

SOME STUDIES ON RURAL WATER SUPPLY SCHEMES

**A Thesis Submitted
in Partial Fulfilment of the Requirements
for the Degree of**

MASTER OF TECHNOLOGY

By

R. P. YADAV

to the

**DEPARTMENT OF CIVIL ENGINEERING
INDIAN INSTITUTE OF TECHNOLOGY KANPUR
MAY, 1980**

DEDICATED TO THE SWEET MEMORY OF
MY PARENTS

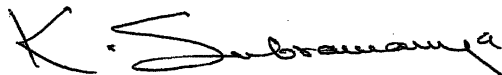
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CERTIFICATE

This is to certify that the thesis 'Some Studies on Rural Water Supply Schemes' submitted by Mr. R.P. Yadav in partial fulfilment of the requirements for the degree of Master of Technology of the Indian Institute of Technology, Kanpur, is a record of bonafide research work carried out by him under my supervision and guidance. The work embodied in this thesis has not been submitted elsewhere for a degree.



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LIST OF SYMBOLS AND ABBREVIATIONS

Symbols

C	Hazen Williams Roughness Coefficient
γ	Specific Weight
γ_w	Specific Weight of Water
S	Specific Gravity

Abbreviations

WHA	World Health Assembly
LSGED	Local Self Government Engineering Department
IDA	International Development Authority
LIC	Life Insurance Corporation
RWSS	Rural Water Supply Scheme
RWS	Rural Water Supply
l.p.c.d.	Litres per capita per Day
A.C.	Asbestos Cement
P.V.C.	Polythene Polyvinyl Chloride
G.I.	Galvanized Iron
C.I.	Cast Iron
B.H.P.	Break Horse Power
UNICEF	United Nations International Children's Emergency Fund
NWSP	National Water Supply and Sanitation Programme
O.H.T.	Over Head Tank

RCC	Reinforced Cement Concrete
KL	Kilolitre
PSIG	Per Square Inch Gauge
MGD	Million Gallons Per Day
GPM	Gallons Per Minute
CFS	Cubic Feet per Second
l/s	Litres per Second
LP	Linear Programming
HGL	Hydraulic Grade Line

ABSTRACT

The rural water supply schemes of U.P. Jal Nigam have been studied by taking five representative projects. The present studies of design criteria and the overall typical characteristics of these water supply schemes have been highlighted. The need for an efficient design and analysis of water distribution networks has been brought out.

A versatile water distribution network system analysis programme has been implemented at I.I.T. Kanpur DEC 1090 system. The five selected rural water supply schemes were analysed through the use of this versatile programme. The deficiencies in the existing schemes and the methods to modify them have been indicated.

A method for the optimum design of tree type water distribution network has been indicated.

1 INTRODUCTION

1.1 GENERAL

Water, like air, is essential to human survival, so it is a must for a welfare government to provide safe and potable drinking water in adequate quantity to its urban as well as rural population. In recent few years there is a progressive effort towards this.

The United Nations Water Conference held at Mar del Plata, Argentina, in March 1977 recommended that the decade (1980-1990) should be designated as the 'International Drinking Water Supply and Sanitation Decade'. The resolution WHA 30.33 concerning preparations for the decade (1980-1990) has been unanimously adopted by the thirtieth World Health Assembly in May 1977. According to this (i) Implementation of National Plans for drinking water supply and sanitation to all urban and rural communities should be increased, (ii) Specific targets should be set up by each country taking into consideration its sanitary, social and economic conditions²⁰. The above is an important resolution and it is but natural that the Central and State Governments of India will give high priority for drinking water supply and sanitation programme, and provide necessary provisions in the annual plans so as to achieve these objectives.

According to the World Health Statistics Report, 1976, about 77 percent of the urban communities and 22 percent of the rural communities among the developing countries have been provided with adequate water supply upto 1975. Progress in urban water supply in India is comparable to that at global and South East Asia regional level, but it is rather poor in rural water supply systems. In India 60 percent of the urban community and 4 percent of the rural community was served by water supply facility in 1970 and 79 percent and 18 percent in 1975 respectively, as indicated in Table 1.1.

TABLE 1.1 WATER SUPPLY POSITION IN INDIA

Community	Population served (percent)		Population to be served (Target) (percent)	
	1970	1975	1980	1990
Urban	60	79	98	100
Rural	4	18	32	100

As per 1971 Census the country had an urban population of 1090.94 million spread in 3,119 towns and rural population of 438.58 million in 5,75,936 villages and hamlets. The population in 3,18,633 villages (55.32 percent) is less than

500. An additional 1,32,990 villages have population in the range of 500-999 making a total of 4,51,626 (78.41 percent). These villages are scattered in the countrysides, some of which are inaccessible during part of the year whereas some have no reliable source nearby. A total of 1,52,475 villages came under the category of scarcity or problem villages, which pose a challenge to those involved in water supply programme.

Water with high chemical ingredients such as fluorides, chloride, iron and manganese have been found in about 25000 villages with a population of approximately 21.93 million. In addition water borne diseases such as dysentery, gastro enteritis, infectious hepatitis, enteric fevers and poliomyelites are prevalent in urban and rural areas. The death rate due to these diseases have been found to be 9.6 and 16.5 per thousand persons in urban and rural communities respectively. This reflects an urgent need of safe water supply and other public health facilities, especially in rural areas.

Out of the total annual precipitation of 400 million hectare meter, only about 105 million hectare meter is estimated to be ultimately utilizable containing of 70 million hectare meter of surface water and 35 million hectare meter of ground water. The surface water is not safe as it gets

polluted due to increased industrialization. In ground water there is presence of chemical ingredients. So, there is a need of some technological solution for safe water supply. These are the reasons why India Government is giving more emphasis on rural water supply schemes, from Fourth Five Year Plan (1969-74). The accelerated water supply programme gave a further boost to rural water supply. About 64,000 out of 5,75,936 villages and 1,890 out of 3,119 towns have been provided with water supply by 1978. The outlay on rural water supply was 0.8 percent of the total plan during the first plan which increased to 1.5 percent in the fifth plan. The annual per capita expenditure on rural water supply and sanitation for the unserved population of the entire country was increased from Re 1.00 from the fourth plan to Rs. 2.4 in the fifth plan (1974-1978).

Several financing institutions like the Life Insurance Corporation of India and Banks have been providing part of funds required for water supply schemes. Funding from international and bilateral agencies are available now.

1.2 U.P. JAL NIGAM

Previously supplying of water to a community was the responsibility of the municipal authorities. But when the work grew in complexity and volume, need for the establishment

of a separate organisation was felt and in 1927, the U.P. Public Health Engineering Department was created. It was later re-named the Local Self Government Engineering Department (LSGED) which is now fully merged in the U.P. Jal Nigam. Further, State Government gave more emphasis on water supply and sanitation which required large initial investment. So, in the seventies the World Bank was approached. But to give financial assistance it wanted a central organisation with statutory powers to be established first, through which loans to the local bodies could be channelized. Due to this the State Government created an organisation named U.P. Jal Nigam on June 18, 1975 by an act of U.P. Legislature. Now it is a dynamic organisation of U.P. Government, which is working under the financial assistance mainly of International Development Authority, (IDA), a subsidiary of the World Bank for soft lending to member nations of United Nations Organisation for development programmes; the Dutch Government and the Life Insurance Corporation of India (LIC)⁷.

It consists of eight Jal Sansthan, three Regional Jal Sansthan namely Garhwal, Kumaon and Bundelkhand and five local Jal Sansthan for KAVAI towns ie. Kanpur, Agra, Varanasi, Allahabad and Lucknow to give proper water supply and sewerage facilities.

During the five Five-Year Plans only 6,670 villages were brought under the piped water supply. Out of the 1,12,561 revenue villages in the state 35,506 villages suffer from scarcity of potable water. Similarly out of the 619 urban localities barely 376 localities had piped water supply. Till the end of the Fifth Five Year Plan only 39 urban localities were provided the sewerage facility. The Nigam provided water to 1,917 villages during the year 1978-79. This compares very favourably with the 6,670 villages which were provided this facility during the last five Five Year Plans, i.e. upto 1978, and that. During the year 1977-78, drinking water was provided to 1,200 villages.

During the Sixth Five Year Plan period 1978-83 it has been envisaged to provide piped water supply to all the 619 urban local bodies of the state as well as to provide underground sewerage system in 39 more towns. Strengthening of water supply has been proposed in 83 towns and sewerage reorganisation in eight towns. A target covering 14,200 more villages has been set for rural water supply programme. To boost the progress of rural water supply schemes, the Central Government has launched a crash programme under the title, 'Accelerated Rural Water Supply Programme' and it is likely that a sum of Rs. 26 crores will be made available for this, besides the plan allocations. This will be utilised

for provision of safe drinking water supply facilities to about 2,250 additional villages.

1.3 DESIGN CRITERIA

The design criteria for the RWSS adopted by U.P. Jal Nigam with recent guidelines is as following:-

Design Period

The design period should be 30 years.

Population

The rural design population should not be estimated by the previous criterion of increasing the present population by 50 percent except the villages bordering the towns where the percentage increase in the design population may be more.

Any one of the following methods is to be adopted to calculate the design population.

- i) Arithmetical mean method
- ii) Geometrical increase method
- iii) Incremental increase method
- iv) Decrease in percentage increase method
- v) Graphical method
 - a) an ordinary plot method
 - b) a semi-log plot method
- vi) Comparative method

Floating Population

The equivalent population for feeding the institution, market and mela may be considered as 20 percent of the total population.

Rate of Water Supply

- (i) 70 l.p.c.d. for villages where individual design population is less than 4,000 and where private house connections are expected.
- (ii) 50 l.p.c.d. for small villages where no or few house connections are expected.
- (iii) 25 l.p.c.d. where adequate source is not available. But the schemes should be framed on 50 l.p.c.d. so that except for 2 to 3 months of the summer adequate quantity of water is available to the beneficiaries.
- (iv) 90 l.p.c.d. for villages where individual design population is more than 4,000.

Source Discharge

The discharges of the source should normally be measured for three consecutive years of the driest season and the least discharge should be adopted. In the case if the scheme is to be prepared urgently, the one year driest discharge should be double than the ultimate requirement.

In special circumstances where there is an acute scarcity of drinking water the scheme can be framed if the discharge of the source can feed at a rate of 25 l.p.c.d.

Service Reservoir

Minimum storage however should not be less than 10 l.p.c.d. or 1/6th of the days requirement whichever is more.

Peak Factor

For KAVAL towns	2.0
For Urban towns	2.5
For Rural areas	2.5
For Industrial areas	1.0

These peak factors are applicable on maximum day per capita rates adopted.

Distribution System

The distribution network should be designed for peak flow of peak factor times the average demand.

The minimum size of a distribution main should be kept according to the size of the town as per following criteria.

Design Population	Minimum Size of Distribution Main
Upto 5,000	50 mm
5001 to 50,000	80 mm
50,001 to 5,00,000	100 mm
above 5,00,000	125 mm

In hills the minimum size shall be 25 mm.

For towns of population above one lac wherever the main road is more than 20 m in width, mains will be laid on both sides so that the road is not cut for service connections later on.

Pipes

Medium quality of G.I. pipes should be used for water supply schemes of hilly areas.

In plains P.V.C. pipes may be adopted upto a dia of 100 mm. A.C. pipes should be used for bigger sizes. Presently upto 150 mm P.V.C., 200 to 600 mm A.C. and above 600 mm C.I. pipes are being used.

Terminal Pressures

Terminal pressures shall be provided as listed in the following tabular shape.

Design Population of the town(in lacs)	Building type	Terminal Pressure (m)
Upto 0.20	Single storied	7
	Double storied	12
0.20 to 0.50		
0.50 to 1.00	No consideration of the height of building	12
1.00 to 5.00		
5.00 to 10.00	No consideration of the height of building	15
above 10.00		

Terminal pressure for villages shall be 7.0 meters. Minimum terminal pressure of 6 m for single storied buildings with market and a few house connections and 8 m for double storied building with a large number of house connections were adopted previously by Nigam.

Pipe Lines

Sluice valve/wheel valve may be provided at a spacing of 21 cms (for sizes greater than 150 mm, the spacing may be reduced as required).

Terminal pressure - Minimum 6 m

Design of distribution system are to be done for each and every village. Station pressure must be duly considered for deciding classes of pipes. Break pressure tables are to be provided as required.

Stand posts considering initial population (upper limit)

- (i) One for about 150 persons in hilly areas
- (ii) One for about 250 persons in plains.

There should be one stand post atleast for weaker section such as Harijan and Tribal community.

Air valve should be provided on summits.

Tank type stand posts, instead of single tap pillar type previously used.

- (i) In such villages where no private connections are

expected in the beginning, tank type stand posts with capacities of 2000, 3000, 5000 litres may be provided, keeping in view 1/2 day's requirement in the initial stages.

- (ii) The distribution system even in these villages will be designed for 6 meters, minimum terminal pressure to provide for house connections in future.

Rising Main

The economical size of rising main should be calculated as per departmental tabular procedure taking electricity tariff rule for the cost of every corporation economics of laying a size main throughout the design period to that of duplicating after 15 years be worked.

For tube wells and other pumping schemes requiring short length of rising main (say not exceeding 200 m), it would be desirable to use C.I. pipes. At other place A.C. pipe tested upto adequate pressure may be considered.

Consumption

The annual average supply of water is expected to be about 75 percent. The consumption of chemicals, electric or diesel should be calculated on this pattern. Minimum of two tube wells may be provided. The pumping hours of the tube well may generally be taken 16 hrs at the ultimate period of the scheme.

Hydraulic Gradient

To design the distribution main, such hydraulic gradient may be adopted for different type of pipe materials as to provide an economical design. For this purpose the hydraulic gradient arrived at for economic design of rising main shall be adopted. Generally the hydraulic gradient for P.V.C. and A.C. pressure pipes shall be 3 to 4 per thousand and for C.I. and steel pipes 5 to 6 per thousand. However the gradient may vary according to the minimum size of the distribution main as recommended in para 'Distribution System'.

Size of Pump House

BHP	Size
10.0	8' x 10' x 12'
12.5 to 40.0	12' x 10' x 12'

The chloronome house is to be constructed jointly with pump house as per type design.

Accommodation of the Staff

- (i) For pump operator - one single room quarter with box room.
- (ii) For chawkidar - one single room quarter with one box room.

Provision for more buildings is to be made after

obtaining Chief Engineer's instruction. For gravity schemes where provision of part-time staff for maintenance purposes is considered adequate, number of buildings should be provided.

List of Various Categories of Pipes and Their Working Pressures

(i) A.C. Pipes

Type of Pipe	Working Pressure
Class I	2.5 kg/cm ²
Class II	5.0 kg/cm ²
Class III	7.5 kg/cm ²

(ii) G.I. Pipes

Size (mm)	Pressure (kg/cm ²)		
	Light	Medium	Heavy
6 to 25	10.5	21.0	24.6
32 to 40	8.8	17.6	21.1
50 to 80	7.0	14.0	17.6
80 to 100	5.6	10.5	14.0
125	-	10.5	14.0
150	-	8.8	10.5

(iii) P.V.C. Pipes

These are available in 4 , 6 and 20 kg/cm² working pressures, M/s Wavin India Ltd. have, however, offered pipes capable of withstanding a working pressure of 5 kg/cm² against 4 kg/cm² pipes.

(iv) C.I. S/S Pipes

Class	Pressure(kg/cm ²)
LA	6
A	9
B	12

Design Formula

Hazen and Williams formula is to be used for the design of the distribution system.

1.4 FINANCE

Considerable emphasis is being given to the RWSS. as laid in the 'International Drinking Water and Sanitation Programme' to make all the villages of the countries provided with the safe drinking water by 1990. Our national Government is also taking keen interest in this field to fulfill the target. But to do this our developing country needs much financial assistance. So this assistance is being taken from International and National financing concerns. For the U.P. Jal Nigam the finance pattern has two ways - Plan and Non-plan. Under plan, the finance is given by IDA and Non-IDA procedures. In IDA procedure the finance comes 50 percent from IDA, an affiliate of the World Bank and 50 percent from the LIC (India). Under Non-IDA, the finance is provided by Central and State Governments through granting and loaning ways, according to the

financial situation of the area concerned, and by the LIC and other Banks through loaning, which is to be returned in few years by the Department. Government gives 100 percent grant to the scheme whose public finance condition is poor and there is no proper source of return of money and 75 percent grant and 25 percent loan or 50 percent grant and 50 percent loan, to the schemes whose area is prosperous. Under the non-plan deposit work comes. If Department contracts some private concern's scheme, the Department needs full cost of the scheme in advance deposited and work will be started after full payment to the Department. To accelerate the National Water Supply and Sanitation Programme (Rural), the financial assistance is to be needed urgently. For this at this time UNICEF and Dutch Government are giving finance.

1.5 NEED FOR AN OPTIMAL DESIGN

The distribution system of a water supply scheme has a major part of the cost incurred on the scheme. So, it should be designed economically. There is no proper and accurate method to design the network system in an optimal manner. But there are more works on network analysis. By this analysis we can optimize the network by changing the diameters of the pipes. Nowadays we have computer facilities available generally in our country, so that through computers we can analyse even a big network within seconds by using an efficient computer

programme. The RWSS where finance is a main problem, can benefit considerably through efficient, optimal designs.

In the present study an analysis of few RWS schemes under implementation of U.P. Jal Nigam have been made by a modified and efficient computer programme to bring out the efficiency and advantages of computer use in such large investment projects.

2 PRESENT STUDY

2.1 GENERAL

The topic of RWS in U.P., for which not only our State or Central Government is taking interest but some other Foreign Governments and International Organisations are giving help, has been selected for the study. The Chief Engineer (Project and Design) and one Research Executive Engineer of U.P. Jal Nigam provided the necessary background and details about the RWS schemes. Reports of three RWS schemes of District Allahabad were made available. Also the Executive Engineer, IInd T.C.D. Kanpur provided valuable informations and details of two RWS schemes of District Kanpur. The details of the various schemes are in the next sections.

2.2 SALIENT FEATURES

A. Girdkot, Zone A, Group of Villages RWSS¹⁴.

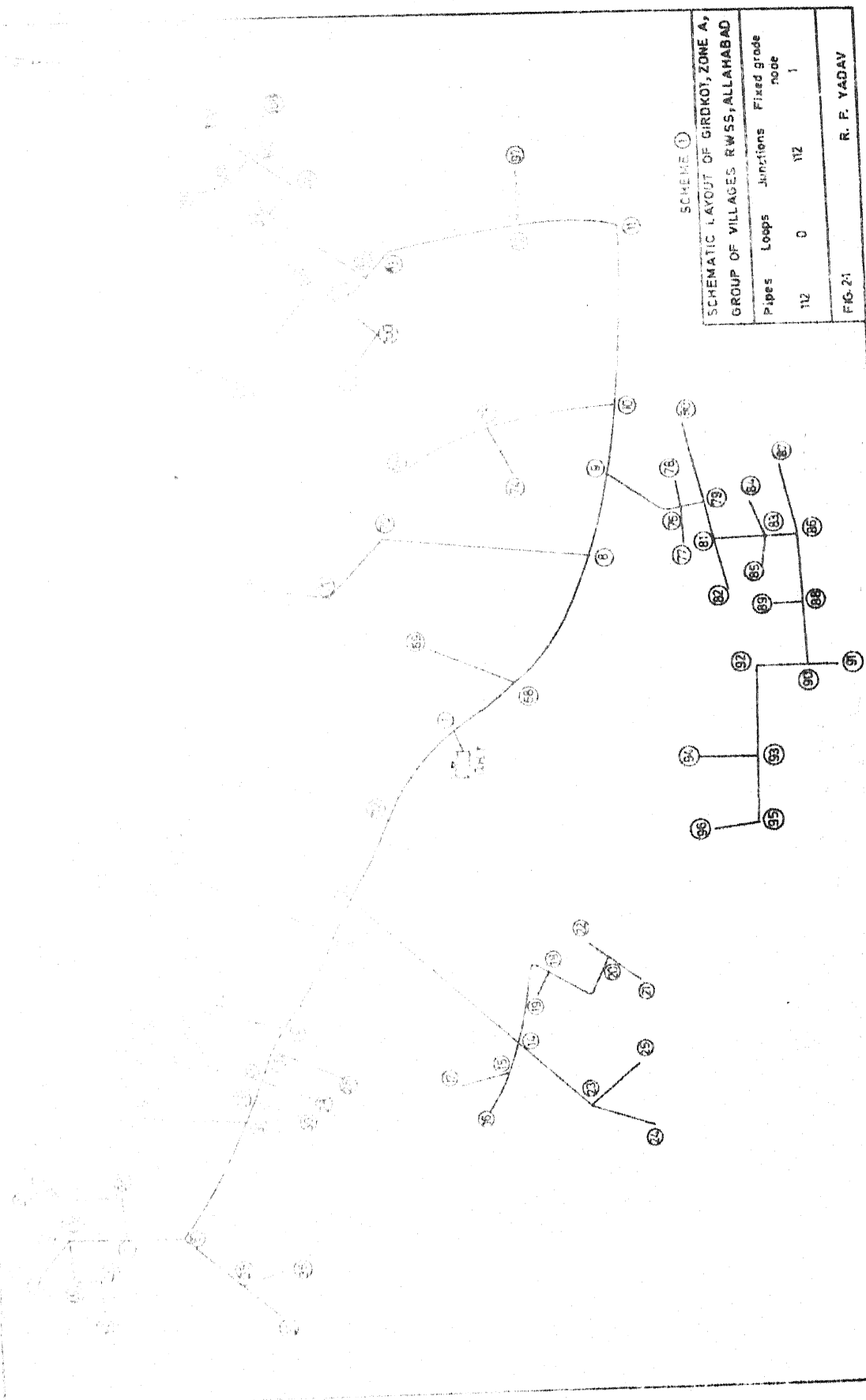
Name of programme	: N.W.S.P. (Rural)
Name of Local Body	: District Board, Allahabad
Number of Villages Covered	: 16
Population	: Present: 7,476 (1971) Designed: 11,214 (2001)
Rate of W/S	: 70 l.p.c.d.
Source of W/S	: Tube well
Nature of Treatment	: Pressure feed type chlorination

SCHEME ①

SCHEMATIC LAYOUT OF GIRDKOT, ZONE A,
GROUP OF VILLAGES RWSS, ALLAHABAD

Pipes	Loops	Junctions	Fixed grade node
112	0	112	1

FIG-21
R. P. YADAV



Conveyance : A.C. and P.V.C. pipes
 Service Storage : One O.H.T. of
 Capacity : 200 KL (based on 1/4th
 of the ultimate
 daily requirement
 of water)
 Staging : 15 m
 Material : RCC
 of cons-
 truction
 Distribution system : Peak factor : 2.4
 Minimum size: 25 mm (internal)
 Minimum terminal pressure: 7 m
 Kind, size and class of pipe

Kind of pipe	Size, ϕ (mm)	Class	C
A.C.	125 to 250	10	130.00
P.V.C.	25 to 100	4 to 10 kg/cm ²	140.00

Estimated Cost : 9.00 lacs
 Pumping plant : Plant capacity: 25 BHP
 Pumping Hours : 10
 Draw off rate of balancing reservoir : 16 Hours (standard)

This scheme comes in Handia Tehsil of Allahabad District.

The area under Handia Tehsil is such that there is no river or any other natural source to get drinking water in abundant quantity. It is very difficult to get water. In summers the draught condition is a regular feature. The ground water

condition is good here, so this water is to be utilized for drinking. This rural water supply scheme has been drawn up for the villages which are draught affected and the villages which come in the alignment of pipe lines for draught stricken villages.

Taking the design period 30 years, the designed population at the end of the design period i.e. in the year 2001 has been taken as 11,214 assuming 50 percent increase over the 1971 population taking into account the future growth and development.

To make the scheme economical it was divided into zones as follows:

Zone	Number of villages	Population designed
A	16	11,214
B	21	11,814
C	21	14,590

B. Ketehra, Zone A, Group of Villages RWSS¹⁵

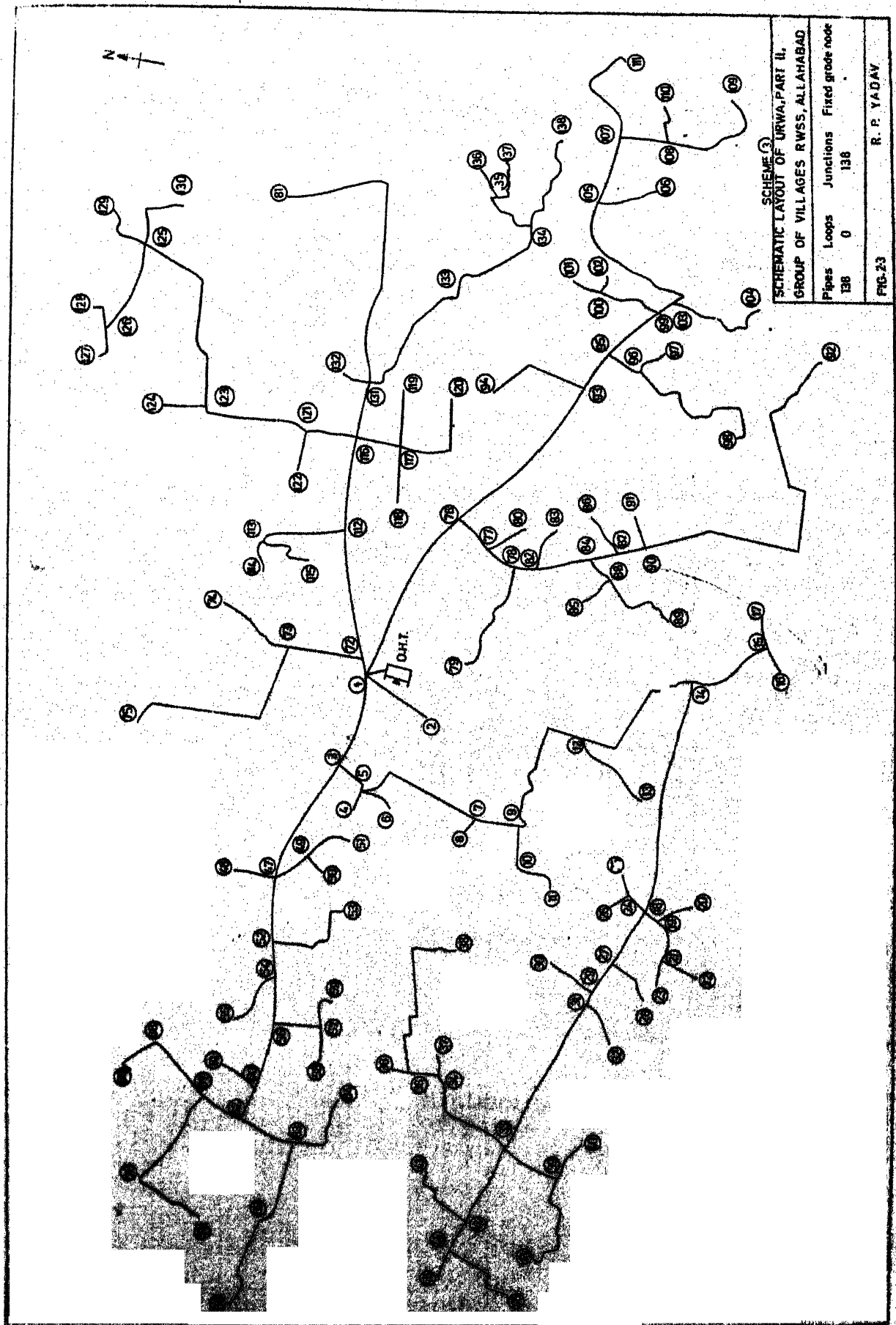
Name of the programme : NWSP (Rural)

Name of the Local body : District Board, Allahabad

Number of villages covered : 13

Population : Present:7,123 (1971)
Designed:10,693 (2001)

Rate of W/S : 68.1 l.p.c.d.



Nature of source : Tube well
 Source of Development : Pump House
 Nature of treatment : Pressure feed type chlorination
 Conveyance : A.C. and P.V.C. pipes
 Service storage : One RCC O.H.T. of
 Capacity: 120 KL (based on 1/6th of
 the ultimate daily
 requirement of water)
 Staging : 17 m
 Distribution system : Peak demand factor: 2.4
 Minimum terminal head: 7.0 m
 Minimum size : 25 mm
 kind, size and class of pipe

Kinds of pipes	Size, ϕ (mm)	Class	C
A.C.	125 to 250	II	130.00
P.V.C.	25 to 100	-	140.00
G.I.	15 to 20	Light quality	-

Estimated cost : 9.0 lacs
 Pumping plant : Plant capacity: 20 BHP
 Pumping Hours: 10
 Draw off rate of balancing reservoir : 16 Hours (standard)

This scheme also comes in Handia Tehsil of District Allahabad. Due to absence of river and any other natural source the ground water is to be supplied for drinking to the draught

stricken, Ketehra group of villages, the condition of which is good.

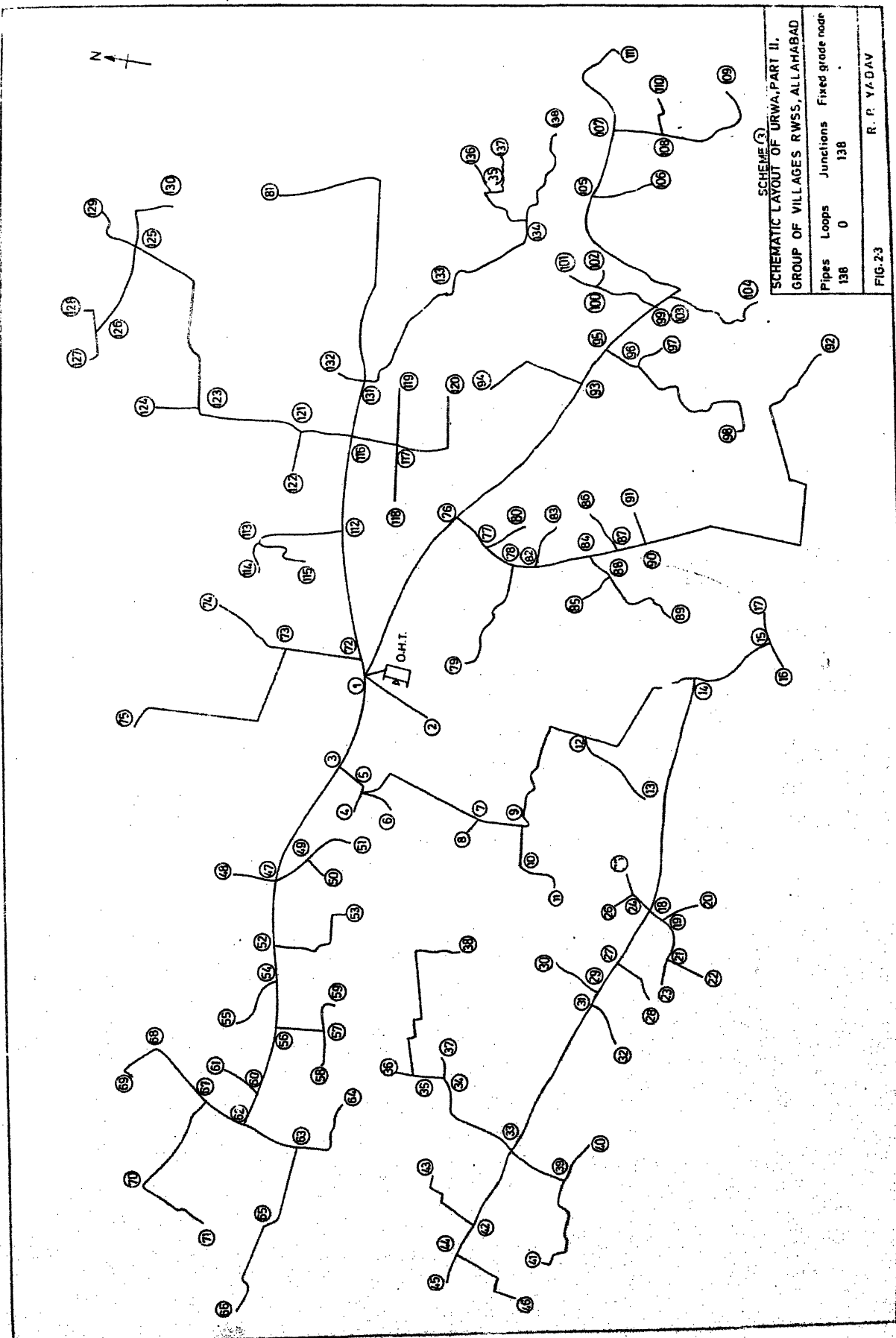
As per 1961 Census, the population of all the 52 villages covered by this scheme was 19,643. The estimate of this scheme was originally prepared by scarcity division, Mirzapur in the year 1972-73 and the population of 1971 Census was taken as the initial population by increasing the population of 1961 Census by 25 percent which comes to be 24,541. This was assumed that over a period of 30 years further increase in population would be 50 percent keeping a margin for family planning measures and migration of rural population to urban areas, including floating population due to occurrence of cattle fair and religious festivals.

To make the scheme economical it was divided into three zones as follows:

Zone	Number of villages	Population designed
A	13	10,693
B	20	16,593
C	19	9,533

C. Urwa, Part II, Group of Villages RWSS¹⁶

Name of programme : Dutch programme
 Name of local body : District Board, Allahabad
 Number of villages covered : 23



Population : Census, 1971 : 16,836
 Present, 1981 : 20,246
 Designed, 2011 : 30,369

Rate of W/S : 70 l.p.c.d. for population < 4,000
 90 l.p.c.d. for population > 4,000

Source of W/S : Tube well

Nature of treatment : Chlorination by differential pressure feed type chlorinating plant

Conveyance : Forced main of C.I. (class, LA, C=100.00) S/S pipe of

Ø(mm)	Length(m)
200	300
300	50

Service storage : One O.H.T. of R.C.C. having
 Capacity: 650 KL (based on 1/4th of the ultimate daily requirement of water)
 Staging : 14m

Distribution system : Peak demand factor : 2.4
 Terminal head: 6m for single storied building with few house connections
 8m for double storied building
 Minimum size of pipe: 25 mm

Kind, size and class of pipes

Kind	Size, ϕ (mm)	Class	C
A.C.	200 to 250	10	130.00
P.V.C.	25	10 kg/cm ²	140.00
	32 to 40	6 kg/cm ²	140.00
	50 to 150	4 kg/cm ²	140.00

Appurtenances	:	Sluice valve	:	52 nos.
		Wheel valve	:	4 nos.
		Air valve single ball screwed down type 20 mm	:	12 nos.
		Fire hydrant	:	15 nos.
		Single tap type Stand post	:	42 nos.(30 percent for scheduled cast localities)
Average dose of chlorination	:	0.5 PPM		
Estimated cost	:	39.23 lacs		

This scheme falls under Tehsil Meja, District Allahabad. Scarcity of drinking water is a regular feature in summer in this area. To face the draught conditions in every summer of the area a permanent piped water supply has been proposed. The railway line from Allahabad to Mirzapur goes across through this village group. In the southern side of the railway line there is URWA, Part I, group of villages RWSS which has already been implemented. In the north side of the same this scheme

is to be implemented.

The base year for the design period is 1981. As per Census figures the growth of population in the district is 20.4 percent for 1961 to 1971 decade; so to arrive at the population for 1981, the 1971 population has been increased by 20.4 percent. The ultimate population has been calculated for the year 2011 anticipating an increase of 50 percent. Over the 1981 population.

Two centrally located tube wells with pumping plants designed for discharge of 2000 lpm, at a head of 40 meters with 25 HP motor running 6.5 hours each in the beginning, 8 hours in the middle and 9.5 hours at the end of the design period shall be sufficient to cope with the need of scheme.

D. Malasa Group of Villages RWSS¹⁷

Name of the programme : NWSP (Rural)

Name of the local body : District Board, Kanpur

Number of villages covered : 15

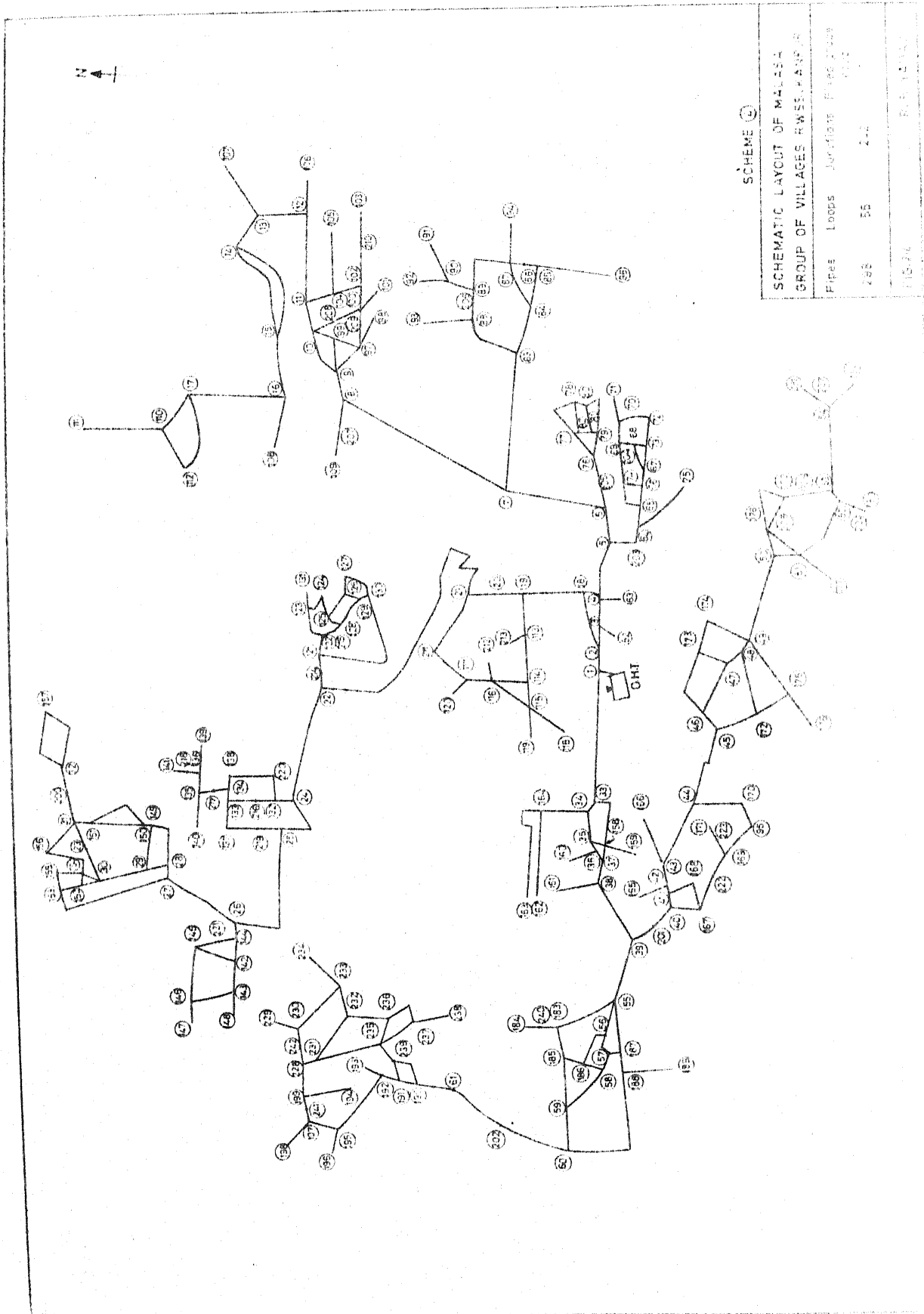
Population : Present : 15,400 (1978)
Designed: 23,100 (2008)

Rate of W/S : 70 l.p.c.d.

Nature of source : Tube well

Source of Development : Pump house

Nature of Treatment : Gaseous injector type chlorination using chlorine gas



- Conveyance : Forced main working on maximum pressure of 60 m water head of C.I. (class LA) having 200 mm internal dia and 330 m length.
- Service storage : One O.H.T. of RCC of
Capacity : 500 KL (based a 1/4th of the ultimate daily requirement of water)
Staging : 20 m
- Appurtenances : Sluice valves : 40 nos.
Wheel valves : 35 nos.
Air valves : 5 nos.
Scour valves : 5 nos
Public stand posts
Proposed single tap
pillar type : 75 nos. (18 nos. are for weaker section).
- Distribution system : Peak demand factor : 2.4
Terminal' pressure : Min. 6m head
Minimum size of pipe : 25 mm
Kind, size and class of pipes
- | Kind | Size, Ø(mm) | Class | C |
|--------|-------------|-------|--------|
| A.C. | 125 to 250 | II | 130.00 |
| P.V.C. | 25 to 100 | - | 140.00 |
- Dose of chlorination : Maximum : 2.0 PPM
Average : 0.5 PPM
Range : 0.1 to 2.0 PPM
- Estimated capital cost : 25.48 lac cs

Pumping plant : Two electrically driven oil lubricated vertical base hole turbine pumping plants

Plant capacity : 35 BHP

Pumping Hours : 16

Draw off rate of balancing reservoir : 16 hrs (standard)

Number of tubewells : 2 (one standbye)

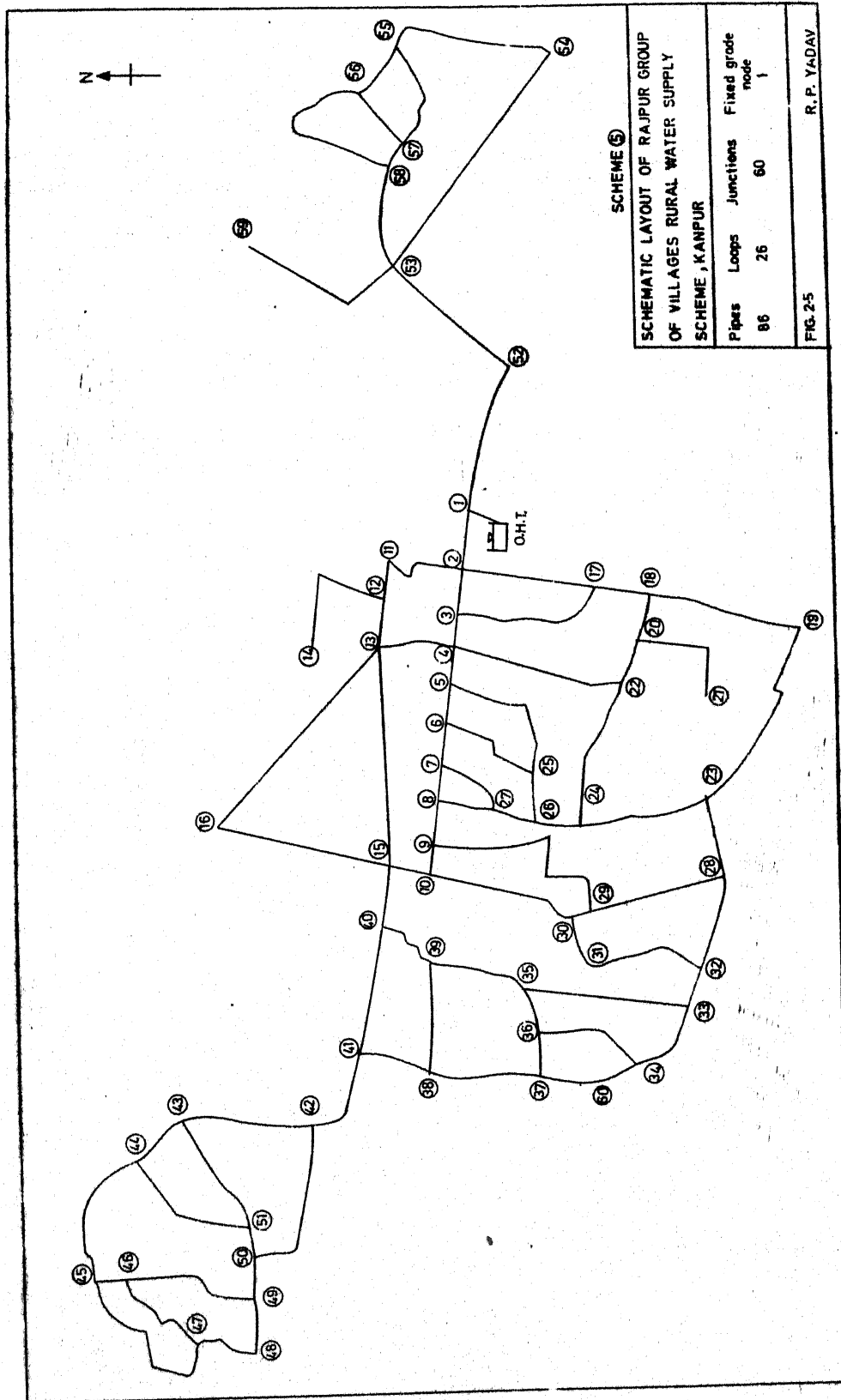
The villages draw their daily requirement of water for drinking and domestic purposes from open wells and hand pumps which are inadequate in number and are liable to contamination. The available water of the wells in some of the villages is brackish as well as unhygienic which is not advisable for human use. Besides this the ground water level in this area is considerably low varying from 19 to 25 meters below ground level. So, piped water supply becomes essential in this area.

The 1971 Census population of the proposed group of villages is 13,883. The population in the beginning of the design period (1978) works out to be 15,400 by increasing the 1971 Census population by 12 percent. The design population after a period of 30 years (2008) works out to be 23,100 after increasing the initial population by 50 percent.

E. Rajpur Group of Villages RWSS¹⁸

Name of the programme : NWSP (Rural)

Name of local body : District Board, Kanpur



Number of villages covered: 3

Population : Present: 6,439 (1976)

Designed: 10,000 (2006)

Rate of W/S : 70 l.p.c.d. for population <4,000
90 l.p.c.d. for population >4,000

Nature of source : Tube well

Source of Development : Pump house

Nature of treatment : Pressure feed type chlorination

Conveyance : Forced main of C.I. (class LA)
having

size ϕ (mm)	length(m)
------------------	-----------

125	400
-----	-----

150	30
-----	----

Service storage : One RCC O.H.T. of

Capacity: 135 KL (based on 1/6th
of the ultimate daily
requirement of water)

Staging : 12 m

Distribution system : Peak demand factor : 2.4

Minimum terminal pressure: 6 m

Minimum size: 32 mm

Kind	size ϕ (mm)	Class	C
A.C.	65 to 200	II	130.00
P.V.C.	32 to 50	-	140.00

Appurtenances : Sluice valve : 15 nos.

Air valve : 2 nos.

Fire hydrant : 4 nos.

Standposts : Single tap pillar type : 10 nos.

Dose of chlorination : Maximum : 2 PPM

Average : 0.5 PPM

Estimated capital cost : 7.05 lacs

Number of tube wells : Two (one standbye)

Pumping plant : Plant capacity : 7.5 BHP

Pumping Hours: 14

Draw off rate of balancing reservoir : 16 hrs. (standard)

The open wells are only the sources of drinking water in this area, which are liable to be contaminated and in summer draw inadequate water. Water table of the area is nearabout 20 m deep. So, piped water supply has been necessitated.

As per 1971 Census, the population of proposed group of villages is 5,699. The initial population (1976) works out to be 8,439 after allowing increase of 2.7 percent per year. Population after 30 years (2006) works out to be 10,000 after increasing the initial population by 50 percent.

2.3 COSTS FOR EFFICIENT DESIGN

The water supply distribution networks of the five schemes were analysed at the I.I.T. Kanpur computer facility by an efficient computer programme. After analysis it was found that the some schemes were overdesigned i.e. Ketehra and Rajpur.

The pressure heads at the terminal point are much more than that specified in design criteria. Some schemes were deficient and the pressure heads at the terminal points are much less than the specified, i.e. Girdkot, Urwa and Malasa. So, keeping in mind the minimum requirement of the pressure heads at the terminal points, the diameters of some pipes were changed according to the condition, to get more pressure head the diameter was increased, and final costs of the schemes were calculated. Table 2.1 shows the details of costs and savings.

TABLE 2.1: COST AND SAVING DETAILS OF THE SCHEMES

Schemes	Initial Cost (Rs.)*	Final Cost (Rs.)*	Saving (Percent)
A	3.42	3.44	-0.58
B	3.12	3.11	+0.32
C	15.00	15.26	-1.73
D	9.13	9.39	-2.85
E	1.78	1.57	+11.80

* In lacs

2.4 COMPARATIVE STUDY

TABLE 2.2 : COMPREHENSIVE ABSTRACT OF COST

Items	Schemes					
	A.1**	B.1***	C	D	E	
1	2	3	4	5	6	
Distribution system	14,47,600	15,12,000	19,30,111	11,80,000	2,25,000	
Over head tank	3,70,000	2,96,000	3,20,000	2,32,000	80,000	
Pumping plants	1,79,000	1,78,862*	1,34,000	1,40,000	40,000	
Pump House and chlorinating room	33,000	29,000	22,000	14,000	10,000	
Staff building	1,31,600	1,68,000 ⁺	87,000	22,500	-	
Rising main	14,600	14,000	58,000	48,000	22,800	
Boundary wall, approach road and gate	47,200	-	20,000	7,500	1,08,000	
Electric transmission line	48,100	55,300	2,42,500	90,000	20,000	
Chlorinating plant	9,000	-	6,000	14,000	4,000	
Tubular shed	20,000	20,000	3,600	5,400	2,400	
Purchase of special T and P	71,000	71,000	29,000	15,000	-	

Table contd.....

Table 2.2 contd....

1	2	3	4	5	6
Extra cartage of material	5,000	-	-	8,500	4,000
Land acquisition	-	5,000	7,500	-	-
Water meters	-	-	33,060	29,600	10,000
Internal electrification	-	-	-	4,000	-
Provision of work required for diverting the surplus water- for agricultural use	-	-	-	5,900	-
	23,76,100	23,49,162	28,92,771	18,16,400	5,26,200
Total (Rs. in lacs rounded off)	23.76	23.49	28.93	18.16	5.26

* Cost includes for pumping plants plus chlorinating plant

+ Cost includes for staff building plus boundary wall, approach road and gate

** Girdkot group of villages RWSS , Allahabad

*** Kethra group of villages RWSS, Allahabad

TABLE 2.3 : COSTS OF OVER HEAD TANKS

Sl. No.	Schemes	Sta- ging (m)	Capa- city (KL)	Cost (Rs.)	Year	Cost esti- mated in 1980(Rs. in lacs)
1.	A	15	200	1,23,333	1978	1.36
2.	B	17	120	86,500	1978	0.95
3.	C	14	500	3,20,000	1979	3.36
4.	D	20	500	2,32,000	1976	2.82
5.	E	12	135	80,000	1975	1.02

Average cost per 100 KL = Rs. 0.654 lacs

TABLE 2.4: ECONOMICS

Items	Schemes					Average
	A.1*	B.1**	C	D	E	
	1971 (2001)	1971 (2001)	1981 (1996) (2011) ^x	1978 (2001) (2008) ^x	1976 (2006) -	2001
Total population	25,069 (37,618)	24,541 (36,819)	20,246 (25,307) (30,369) ^x	15,400 (21,520) (23,100) ^x	6,439 (10,000) -	
Total capital cost (Rs.)***	30.00 (30.00)	29.95 (29.95)	29.23 (29.23) ^x (29.23)	25.48 (25.48) ^x (25.48)	7.05 (7.05) -	
Total annual maintenance cost (Rs.)	86,000 (1,33,800)	78,000 (1,13,000)	86,000 (1,07,900) ^x (1,29,800) ^x	1,45,000 (1,60,000) ^x (1,11,820) ^x	35,700 (27,200) -	
Cost of scheme per capita (Rs.)	119.66 (79.75)	122.03 (81.34)	194.00 (155.01) ^x (129.00) ^x	165.45 (118.40) 110.00	110.00 (71.00) -	101.10
Maintenance cost per capita (Rs.)	3.43 (3.56)	3.18 (3.07)	4.25 (4.26) ^x (4.27) ^x	9.45 (7.43) ^x (4.84) ^x	5.54 (2.72) -	4.21
Water cost per KL (Rs.)	0.13 (0.14)	0.13 (0.13)	0.21 (0.21) ^x (0.21)	0.37 (0.29) ^x (0.19) ^x	0.22 (0.11) -	0.18
Total cost of scheme/1000 of population(2001)=Rs.1.05***						

* Girdkot group of villages RWSS, Allahabad

** Ketehra group of villages RWSS, Allahabad

*** In lacs

2.5 TYPICAL FEATURES OF A RWSS

According to the comparative study of the economics of the five U.P. Rural Water Supply schemes we get an average total of expenditure incurred on scheme on per capita basis. Per capita average total of expenditure on scheme will be equal to the sum of average per capita capital cost of the scheme and average per capita maintenance cost. The total for the year 2001 comes as Rs. 105.31. This per capita cost of the scheme is helpful for one to know the cost of the scheme, when population is known. One can easily get a total cost of the RWSS in a particular year, when the population to be served is known for that year, allowing a provision of appropriate annual compound interest (say 5 percent).

By the comparative study of over head tanks for all RWS schemes, the costs of the over head tanks for the year 1980 have been calculated by giving a provision of 5 percent annual compound interest. We have got an average total expenditure on over head tank on/100 KL basis, as Rs. 65,400.00. So, we can easily estimate the expenditure incurred on over head tank in an particular year of construction, if we know the capacity of over head tank.

It has been found from the present study that the Over Head Tank for each RWSS was designed and constructed separately. It incurs extra expenditure for its design for each scheme. It also creates difficulty in construction due to changes in each O.H.T.'s, so the working efficiency for the staff for construction becomes low giving more expenditure on scheme. Therefore, it is a need for standardization of O.H.T.'s. There can be a few O.H.T.'s. When we will go to design the distribution system, we can choose a standard design of O.H.T. of standard capacity to suit the scheme.

From the analysis of the network system of the schemes, it has been found that the required terminal pressure heads at some terminal points were not satisfied. There is no accurate method of the analysis in the Department. Manual analysis is very difficult. So, there is a need of distribution analysis by computer. With this we can modify the distribution system to get a nearly optimized network.

Also there is a need of standard features for staff buildings, approach roads and stores etc.

3 ANALYSIS OF WATER DISTRIBUTION SYSTEM

3.1 GENERAL

A few RWS schemes have been taken from U.P. Jal Nigam to get their water distribution networks analysed as discussed in previous chapter. These schemes are traditionally designed and there is no suitable and accurate method in the Department to analyse the schemes. While urban water supply schemes receive considerable attention, in terms of efficient analysis and design, there appears to be no such attempts towards optimal design in RWSS, even though the cost involved is in no way a small amount. So, it is essential to analyse the rural water supply distribution network to meet the flow and pressure requirements and to make the scheme optimal as far as feasible.

3.2 NETWORK ANALYSIS PROGRAMME

An efficient computer programme for the analysis of pressure and flow in pipe distribution systems, prepared by University of Kentucky, was available with Dr. K. Subramanya. This programme was suitably modified and implemented on the IIT Kanpur computer system, DEC 1090. This programme gives rapid and accurate results in only few trials. Features of this programme are given in Appendix I. The computer programme is written in FORTRAN IV, G level. It consists of a main

programme and five sparse matrix subroutines to solve the p linearized simultaneous equations where p is the number of pipes in the system. Before going to the programme a few terms related to distribution network are being defined.

(i) Node

The end points of pipes are called nodes.

(ii) Junction Node

It is a node where two or more pipes meet or where flow is put in or removed from the system. If a pipe diameter changes occurs at a component such as a valve or a pump, this point is a junction node.

(iii) Fixed Grade Node

It is node in the system where both the pressure and elevation (or hydraulic grade line i.e. piezometric head) are known. This is usually the surface of a storage tank or reservoir or a source or discharge point of specified pressure. Each system must have at least one fixed grade node.

(iv) Primary Loop

It is a closed pipe circuit with no closed pipe circuits contained within it.

If the junction, primary loops, and fixed grade nodes are identified as described here the following holds for all pipe system:

$$p = j + l + t - 1 \quad (3.1)$$

where, p = number of pipes

j = number of junction

l = number of primary loops

and t = number of fixed grade nodes.

(v) Pipe System Components

Pipes:

The length, inside diameter and roughness of each pipe must be input as data. One can use roughness coefficient both for Hazen Williams equation and Darcy-Weisbach equation.

Pumps:

A pump can be included in any line of the pipe system. The characteristics of the pump can be input in two ways :

- i) The power the pump puts into the system (in Horse power or kilowatts)
- ii) Minimum three points of operating data (Head-discharge) under normal operating range. This program is designed to work also if the points are outside the normal operating range. A second order curve can be fit to this data to obtain a pump characteristic curve describing the pump operation of the forms

$$E_p = A + BQ + CQ^2 \quad (3.2)$$

where A, B, C, are the characteristic coefficients
and E_p is the head corresponding to the discharge Q.

Minor Loss:

A number of components in a pipe system (such as valves, junctions, bends, meters etc.) produce a head loss, calling minor loss. It could be input easily

$$H_{LM} = M \frac{v^2}{2g} \quad (3.3)$$

where M = minor loss coefficient

V = line velocity

and g = acceleration due to gravity.

Check Valve:

These valves allow flow only in the specified direction. If flow reversal occurs the valve shuts and the line causes no flow.

Pressure Regulating Valves (PRV's):

These valves are designed to maintain a specified discharge pressure (PR) which is lower than the pressure upstream from PRV.

3.3 FORMULATION FOR THE ANALYSIS

3.3.1 Basic Equations

Equation (3.1) giving the relationship between the

number of pipes, loops, junctions, and fixed grade nodes becomes significant when formulating a proper set of hydraulics equations to describe a pipe system.

In terms of the unknown discharge in each pipe, a number of continuity and energy equations can be written equating the number of pipes in the system.

i) Continuity Equation:

For each junction node the flow into the junction should be equal to the flow out of the junction, written as follows:

$$Q_{in} = Q_{out} \quad (j \text{ equations}) \quad (3.4)$$

ii) Energy Equations:

For each loop the sum of head loss around a loop should be equal to the sum of the energy put into the loop liquid by a pump. So for each loop the energy equation can be written as follows:

$$\sum h_L = \sum E_p \quad (1 \text{ equations}) \quad (3.5)$$

where h_L = head loss in each pipe (including minor loss)

E_p = energy put into the liquid by a pump.

If there are no pumps in the loop then the energy equation will state that the sum of the head loss around a loop equals zero.

If there are t fixed grade nodes, $t-1$ energy equations can be written for paths between any two fixed grade nodes as follows:

$$E = \sum h_L - \sum E_p \quad (t-1 \text{ equation}) \quad (3.6)$$

where E = grade difference between the two fixed grade nodes.

Any path in the pipe system can be chosen between these nodes. However, care must be taken to avoid redundant paths. The best method to avoid this difficulty is to either choose all paths starting at one source (like A-B, A-C, A-D, etc.) or to use the previous end point for a path as the starting point for the next path (like A-B, B-C, C-D, etc.). Either of these methods will result in $t-1$ equations with no redundant ones. These junction equations (continuity equations) and loop and path equations (energy equations) constitute a set of simultaneous nonlinear equations which can be solved for the discharge in each line.

3.3.2 Direct Solution of Linearized Equations

Because of the nonlinear nature of the above equations a direct solution is not possible. A linearization procedure is used to handle the non-linear head loss and pump terms so the system of equations can be cast as a set of p linear simultaneous equations which can be solved by routine matrix methods. Essentially the technique used to solve the system equations is this.

Based on an assumed flow in each line (a velocity of 4 units in each pipe is used) the non-linear hydraulic equations are linearized and the linearized equations are simultaneously solved for the flowrates. This set of flowrates is used to linearize the equations and a second solution is obtained. The procedure is repeated until the change in flowrates obtained in successive trials is insignificant. Because all flows are computed simultaneously, convergence is assured and occurs very fast compared to other procedures. Usually only 4-6 trials are required even for large systems. In the present study for the large system, 2 trials were required in tree type configuration and 6 trials in looped type configuration.

3.4 COMPUTER FLOW DIAGRAM

The simplified computer flow diagram has been given on page 49 .

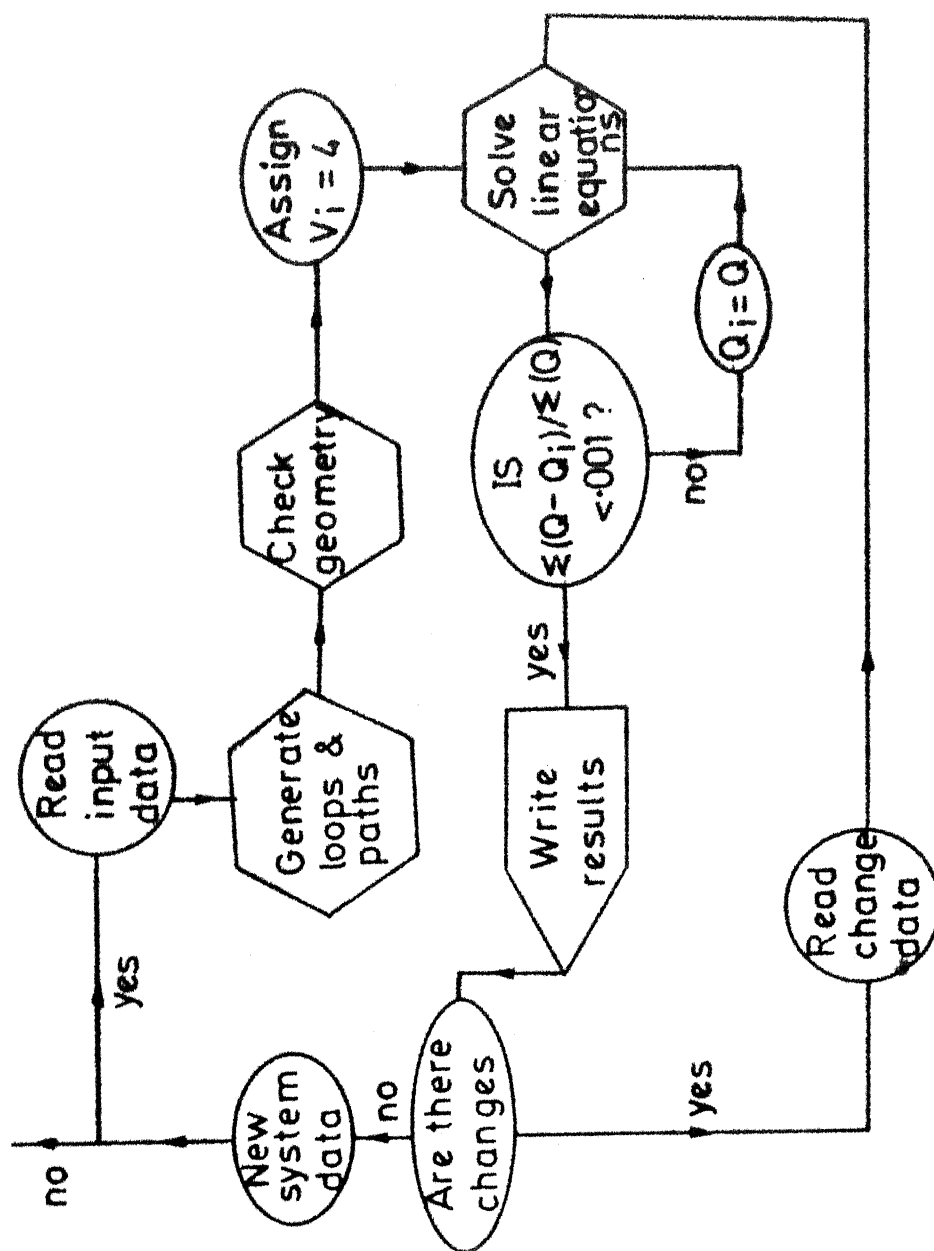


FIG.3-1 SIMPLIFIED COMPUTER FLOW DIAGRAM

3.5 BASIC EQUATIONS

The line head loss is given by

$$h_{LP} = K_P Q^n \quad (3.7)$$

where K_P = pipe line constant

Q = discharge in the pipe line

and n = exponent

For Hazen Williams equation

$$K_P = \frac{K_1 L}{C^{1.852} D^{4.87}} \quad (3.8)$$

and $n = 1.852$

where L = pipe line length

C = Hazen Williams Roughness coefficient

D = internal pipe diameter

and K_1 is a coefficient

= 10.69 for SI units

= 4.73 for English units

For Darcy Weisbach Equation

$$K_P = \frac{K_2 fL}{D^5} \quad (3.9)$$

and $n = 2.0$

where f = friction factor

L = pipe line length

D = internal pipe diameter

and K_2 is a coefficient

= 0.08265 for SI units

= 0.02517 for English units

Minor losses are given by a loss coefficient, M , which multiplies the velocity head to give the loss at the component.

This is

$$h_{LM} = M \frac{V^2}{2g} \quad (3.10)$$

where V = the mean line velocity

and g = the gravitational constant

In terms of the discharge this is

$$h_{LM} = K_M Q^2 \quad (3.11)$$

$$\text{where } K_M = \frac{K_3 M}{D^4} \quad (3.12)$$

Q = discharge in pipe line

D = internal dia of the pipe line

and K_3 is a coefficient

= 0.08265 for SI units

= 0.02517 for English units

The effect of minor loss will be insignificant on the nodal pressure heads so it has not been considered in the present study.

The pump head is expressed in two ways:

- i) When pump power is given, general equation for pump head is

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$$E_P = \frac{P}{\gamma Q} \quad (3.13)$$

where P = power of the pump in watts

γ = specific weight of the liquid = $\gamma_w S$

Q = discharge in the pipe line

γ_w = specific weight of the water

and S = specific gravity of the liquid

$$\text{So, } E_P = \frac{P}{\gamma_w S Q}$$

Here, for SI units

$$\begin{aligned} E_P &= \frac{1000 P}{9806 S Q} \\ \text{or } E_P &= \frac{0.10197 P}{S Q} \end{aligned} \quad (3.14)$$

and for English units

$$\begin{aligned} E_P &= \frac{550 P}{62.42 S Q} \\ \text{or } E_P &= \frac{8.814 P}{S Q} \end{aligned} \quad (3.15)$$

ii) When operating points for the pump are given, the pump head is

$$E_P = A + BQ + CQ^2 \quad (3.16)$$

where A , B , and C are the coefficients of a parabolic characteristic curve which defines the pump operation in the vicinity of the operating point.

Since this expression is only valid over a specified range it should not be indiscretely employed in an analysis.

The basic energy equation for a loop or a path between fixed grade nodes is

$$(h_{LP} + h_{LM}) = E + \sum E_P \quad (3.17)$$

where E is the energy difference between the fixed grade nodes.

This equation can be linearized with the continuity equations to get a set of p simultaneous linear equations in terms of the flowrates in each pipe using a gradient approximation.

3.6 IMPLEMENTATION OF THE PROGRAMME

This original programme is fairly lengthy involving 1,117 FORTRAN cards. The programme was created on the disk of the DEC 1090 system through the cards, and editing was done through the terminals (tty). After compressing the programme file, it occupies 62 blocks storage on disk. The syntax errors were corrected through the terminals. Considerable amount of time was spent on implementing the programme on the DEC system.

3.6.1 Modification due to Computer System

A large number of changes in the programme had to be made to implement it on the DEC system. The major changes are:

- i) '*2' was removed from the programme and 'REAL*8' and 'REAL' were substituted by 'DATA'.
- ii) In READ and WRITE statements device unit numbers were removed and only format statement number were given, also the 'WRITE' was changed to 'PRINT'.
- iii) The cards were arranged in the following order:
 - (a) Name of subroutine;
 - (b) Dimension cards;
 - (c) Variable declaration cards, REAL and INTEGER;
 - (d) Common cards;
 - (e) DATA statement cards;
 - (f) Programme card.

3.6.2 Additions

Some additions were made to run the programme and to get the desired results for the present study. These are:

- i) Array 'ROWCOL(2)' and 'NAME(5)' were introduced in the dimension statements in the subroutines 'MA18A' and 'MA18B' respectively.
- ii) Subroutine COSTT

This subroutine was added to get the total cost of the distribution network. We can give the rate/m length of the pipe for 20 standard pipe diameters. The cost was calculated nicely to know the cost of the scheme just after each analysis.

By making some changes in the system we will get next new cost of the scheme each time. This was felt to know the cost of the scheme when an round off optimized network was being obtained by the provision of putting changes in the system.

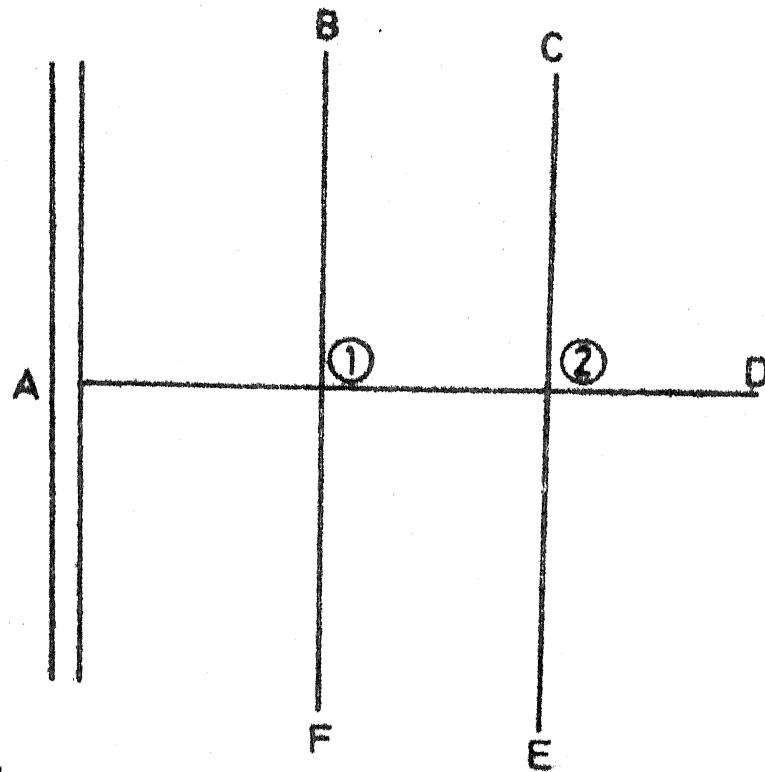
iii) Some FORMAT and PRINT statements were introduced to get the cost output and nice prints out for the U.P. RWS schemes.

3.6.3 Computation Time for a Big Scheme

All the five RWS schemes were analysed on DEC 1090 system. The Malasa Group of villages RWSS is biggest one. The distribution network of this scheme is of loop type. It has 298 pipes. It has been observed that the analysis of loop type network of the same number of pipes takes more CPU time than tree type network, as previous type network takes more number of trials to get the desired accuracy limit. The distribution network of Malasa, RWSS took 7.41 seconds CPU time for its analysis. The initial analysis took 5 trials to get the accuracy of 0.00363 l/s. The analysis after changes takes only 2 trials to get the accuracy of 0.00297 l/s. The execution for this scheme uses CORES of 100P.

3.7 TESTS

The programme was tested for four different types of water distribution networks. The details of the problems



Fixed grades at

A = 253.62

B = 107.87

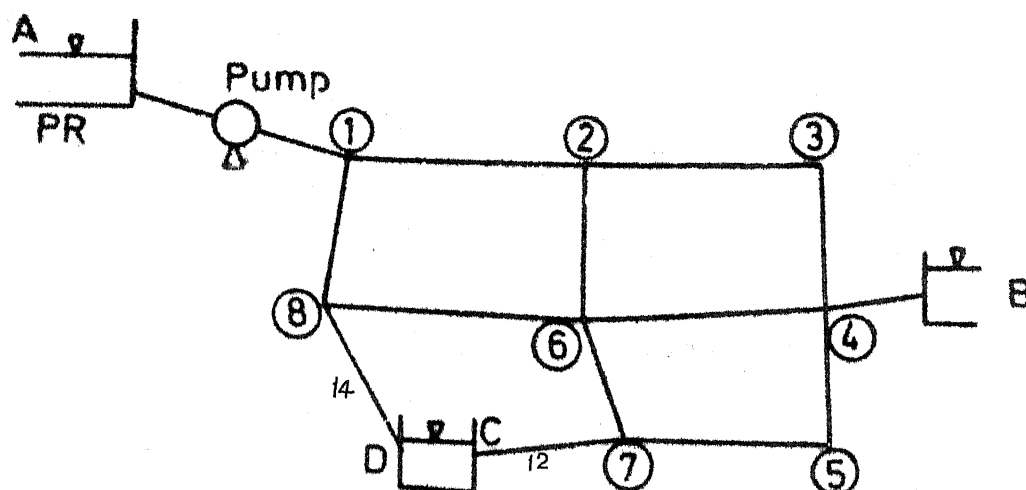
C = 80.90

D = 121.81

E = 43.98

F = 100.00

Fig. 3-2 Schematic diagram of tree type distribution system fed by a large pressure main – English Units, $p=7$, $l=0$, $j=2$, and $t=6$



Fixed grades at

A = 30.05

B = 30.48

C = 33.53

D = 33.53

Pump characteristics

E in (m)	Q (l/s)
165.61	200.00
131.85	600.00
17.83	1000.00

Fig.3.3 Schematic diagram of loop type fourteen pipe distribution system -SI Units, $p=14$, $l=3$, $j=8$ and $t=4$

tested with computer results are as follows:

3.7.1 Test Problem No.1

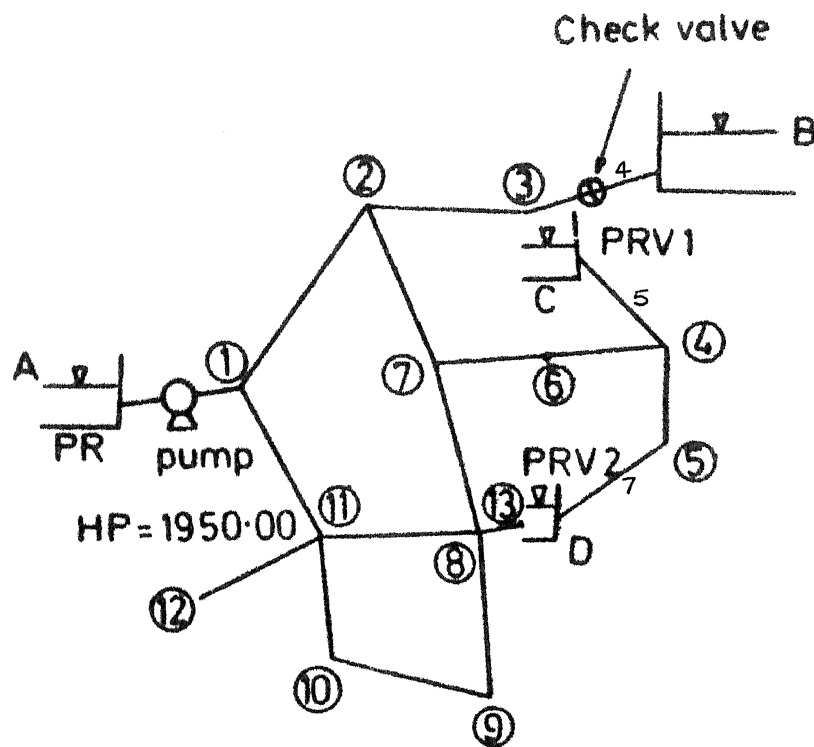
This seven pipe tree type system (Fig. 3.2) is fed by a pressurized main at a constant pressure of 60 psig. All discharge points are fixed grade nodes, labeled A-F. The liquid used is gasoline having specific gravity of 0.68. Here the Darcy Weishbach equation was used in analysis. DEC 10 has taken only 3 trials against 12 trials (previously tested problem) and gave better accuracy limit.

3.7.2 Test Problem No.2

This fourteen pipe system has 3 loops and 4 fixed grade nodes, labeled A, B, C and D (Fig. 3.3). Pipe numbered 12 and 14 both are connected to the same storage reservoir. There is a pump in line 1 and has been described by operating data. Here an option for multiplying factor to the changes for demands is used. The pump operates within the allowable range. It has taken only 3 trials against 4 trials (previously tested problem).

3.7.3 Test Problem No.3

The system has a pump described by the useful horsepower. An item of note is a check valve in line 4, which allows flow only in the direction towards the storage



Fixed grades at

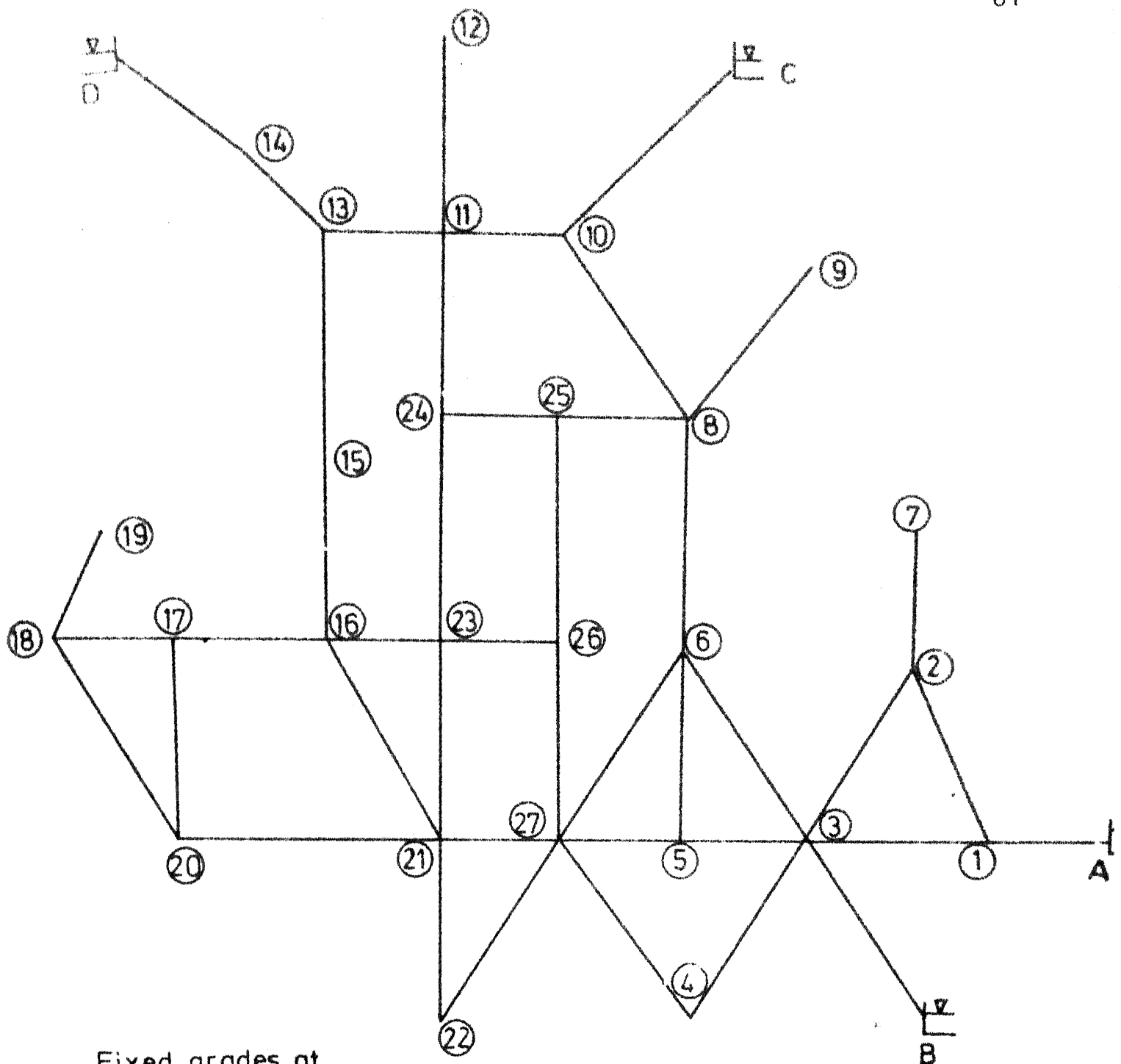
A = 50.00

B = 200.00

C = 303.46

D = 278.46

Fig.3 4 Schematic diagram of pipe water distribution system having one check valve, one pump and 2 PRV's -English Units, $p=18, l=2, j=13, t=4$



Fixed grades at

A = 972.00

B = 935.00

C = 935.00

D = 935.00

Fig.3.5 Schematic layout of improved water distribution system for the city of Montpelier, VT. English Units, $p = 43$, $l = 13$, $j = 27$ and $t = 4$

is 7 m and 11 nodes give less pressure heads. By making changes as shown in the computer print out the scheme has been made safe giving insignificant increase in the cost of the scheme. It took only 2.87 seconds CPU time for this analysis.

3.8.2 Scheme 2

This scheme was found to be over-designed. The terminal pressure were coming more than specified, 7m. It took 1.00 second CPU time. A little cost was reduced after changes to make the network nearly optimal.

3.8.3 Scheme 3

The analysis of this scheme gave more negative pressure at some nodes . One complete branch of the network was under-designed. This 138 pipes tree type network has taken 2.59 seconds CPU time for its analysis. The cost of the scheme has been increased by 1.73 percent to be safe.

3.8.4 Scheme 4

It is the biggest scheme. It comprises 298 pipes and 56 loops. It took 7.73 seconds CPU time. It was more under-designed and closely optimized scheme needed the scheme cost increased by 2.85 percent. Due to loop type it has taken 5 trials for initial analysis.

3.8.5 Scheme 5

It was too much over-designed and after making changes to get nearly optimal network, the saving turned out to be 11.80 percent. It took 2.76 seconds CPU time for analysis.

4 DESIGN OF TREE TYPE DISTRIBUTION SYSTEM

4.1 GENERAL

The essential part of all water supply schemes is the water distribution system. It comprises a considerable part of the total expenditure incurred on water supply schemes. It is apparent from the comparative study of a few U.P. rural water supply schemes, randomly selected, that 65 percent or more of the total cost of the scheme goes to the distribution system. So, optimization of the distribution network is highly desirable.

4.2 PROBLEM

The distribution systems are of mainly two types: (i) loop type and (ii) tree type. As alternate routes are available for transporting water to the demand nodes in looped systems, they are more reliable, though somewhat costlier, than the other type. Reliability is preferred to economy and also the street pattern is grid like for large communities and therefore, loop type networks are employed for urban water supply schemes. For rural water supply schemes, however cost rather than reliability is the design criterion, and also the pattern of villages is of zig-zag nature, so, tree type system is used for them.

Previously all water supply schemes were concerned with Urban communities only. But nowadays worldwide agitation is being made to provide safe drinking water to all rural communities by 1990. In view of vast financial outlay in these projects we have to select optimized distribution network in all schemes.

We can have two ways of optimizing the network systems by (i) global optimal design and (ii) nearly optimized design. Optimal design is really a hard task and can be only possible by an efficient computer programme using the theory of Linear Programming. The other type of design which is to be nearly optimum. This is specifically studied here.

4.3 PAST WORKS

A review of past work in this field is made here before going to the design problem for tree type network systems. Bhav² used the method of critical path approach for the design of dead-end system. He showed that an optimal solution could be obtained directly without trial and error by system reduction procedure and by adopting an economic friction slope. The friction slope is assumed. So, one can not be certain that the optimal solution is achieved, though it may not be far from it. Deb⁹ presented a

method to get optimal solution of the branched pipe network system. He assumed a boundary condition that the total head-loss of the system through any branch is same. Khanna and Swamee¹¹ suggested an optimal design problem which reaches to a non-linear constraints involving an unknown procedure for solution and the work is based on simplifying the problem through approximations. Bhawe¹ has a non-computer approach having steady flows. The approach is based on the critical path method, the method is simple, straight forward and gives a fairly economical distribution method without any trial and error procedure. Deb⁸ suggested that for optimal design of the water distribution system, the initial costs of the pipes, pumps and elevated service reservoir, and maintenance and electricity costs should be considered. According to him the total cost of a water distribution system is dependant on the inlet hydraulic gradient and the pressure surface geometry. With the increase of inlet hydraulic head and the pressure surface parameter (the ratio of the actual slope of the pressure profile at the farthest point from the inflow to the straight line variation slope) value, the total cost decreases initially, reaches a minimum value, and then increases. Optimum values of inlet hydraulic head, H_I and pressure surface parameter, S_R could be obtained. His study also found that the position of the elevated service

reservoir within the network is an important factor in cost optimization. If the position of the service reservoir is removed from the corner of a rectangular pipe network, towards the centre along the diagonal, the distance between the inlet point and the farthest point in the network is reduced, and for a specified head, the hydraulic gradient is increased. As a result, pipe cost is expected to be reduced. It was found that the total cost of the system is found to be 1.33 times more when the reservoir position is at one corner of the network than when it is at the centre.

4.4 CURRENT PRACTICE

From the study of five RWS schemes of the State, U.P., we see that the distribution networks were designed traditionally. In design, the tables based on Hazen Williams Formula were used. As the tables and graphs are used in design, there is a more chance to commit error in selecting a correct pipe diameter for the flow and friction slope conditions. There is no provision to check the error and it will be commulative giving more and more errors. For example, in the design of distribution network of Urwa, Part II, group of villages RWWS, a mistake has been committed in the beginning of a branch having starting node number 78. Due to this mistake the whole branch starting from node number 78 is unsafe from the

terminal pressure head point of view. There is no suitable method to analyse the distribution network after designing it. In other schemes under study, some are over-designed and some under-designed. So, there is an essential need of a proper analysis for the flow and pressure conditions of the network and a proper design procedure.

4.5 SUGGESTED IMPROVEMENT

By the analysis of distribution network we get the terminal pressure condition. Observing the pressure condition we have to change the pipe diameter upstream of the corresponding terminal node, to get the network improved. So, no doubt, the analysis designs the network system in other way.

The computer programme used for the analysis of the scheme here was used to design the network and make it as for as possible optimal. The final costs of the schemes were calculated after the suggested design. The computer programme can be efficiently used to make the system near about optimal which is previously arbitrarily designed. This has been done in Urwa, Part II, group of villages, RWSS, Allahabad.

4.6 A SUGGESTED LP METHOD

4.6.1 General

A method based on the critical path concept can be

developed for selection of the optimal sets of pipe sizes for optimization of branching networks by LP. The number of pipes of consecutive sizes in an optimal set depends upon the quality of optimality needed. Practically two pipe sizes are satisfactory for a system. For global optimality one can select four, or even more pipe sizes. Before going to the critical path concept and behaviour of LP, we will be clear about some definitions.

Path

In branching system there is only one route to each demand node (terminal node) from the source. This route is called path.

Slope of Path

This is maximum available average friction slope.

This is expressed as

$$S = \frac{H_0 - H^{\min}}{L_p} \quad (4.1)$$

where S = slope of the path

H_0 = HGL at the source

H = minimum required HGL at the demand node

and L_p = length of the path

Critical Slope

The minimum of all the path-slopes is termed as critical path.

$$S_c = \text{Minimum} \frac{H_o - H^{\min}}{L_p}, \text{ giving } S_c \leq S \quad (4.2)$$

Critical Path and Critical Node

The path having the maximum available average friction slope equal to the critical slope is termed as critical path and the node at the end of the critical path is termed as critical node.

Link

It consists of one or more pipes connected in series and has a constant flow and no branches.

4.6.2 LP Model

Decision Variable

As the resistance of a pipe and its cost are linear functions of its length, the different pipe lengths constituting a link are taken as the decision variables.

Objective Function

The objective function is to minimize the sum of the capital costs of the various links.

Constraints

(i) For all links the sum of the lengths of the pipes selected must equal the length of the link.

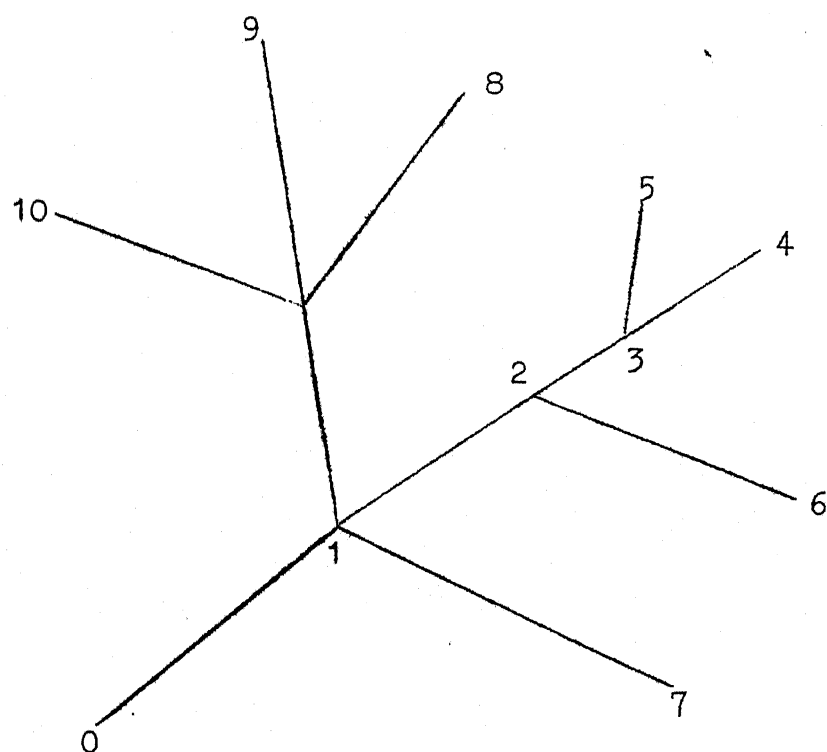


FIG. 4.1: BRANCHING DISTRIBUTION SYSTEM

(ii) At each demand node the available HGL value must be equal to or greater than the minimum required HGL value at the node.

(iii) All pipe-lengths must be non-negative.

By experience, a designer, who carries out the network optimization, selects generally an optimal set of two or three pipe sizes, but this set may not constitute the optimal set. So, critical path concept is suggested here to select optimal set of pipe sizes.

4.6.3 Critical Path Concept

Here, the critical path for the entire system is located. There will be so many paths as demand nodes. The critical one is chosen in the system. For a node i on the critical path, the proposed HGL value H_i will be given by

$$H_i = H_o - S_c L_{Pi} \quad (4.3)$$

From equation (4.1)

$$H_i^{\min} = H_o - S_i L_{Pi} \quad (4.4)$$

From equations (4.3) and (4.4)

$$H_i \geq H_i^{\min}$$

which satisfy the node HGL constraints of the optimization problem, for all the nodes on critical path.

Thus , when all the links on the critical path are provided with critical slope, the node HGL constraints for all the nodes on the critical path are satisfied, and the flow takes place along the other paths.

After deciding the critical path and estimating the HGL values for the nodes lying on it, the critical path is removed from the distribution system. This leaves behind distribution sub-systems of the first order. These distribution sub-systems emerge from the nodes of known HGL values and thus are treated as independent distribution subsystems for which critical subpaths are located and the HGL values for nodes lying on them are estimated. The procedure of obtaining the distribution subsystems of first and higher orders, locating the critical subpaths and estimating the HGL values for nodes lying on them is continued until the proposed friction slopes for all links of the entire system can be estimated.

Practically the commercial pipe sizes which have friction slopes equal to the immediate lower and higher values constitute the optimal set for each link.

4.6.4 Formulation

After getting the critical path, the diameter of all the links lying on that path will be calculated by

$$D = \left(\frac{10.69 Q^{1.852}}{C^{1.852} S} \right)^{1/4.87}$$

where Q = pipe discharge, $m^3/sec.$

C = Hazen Williams constant

S = friction path slope

and D = internal diameter of the pipe in m

The set of two commercial pipe sizes having immediate lower and higher values than the calculated size will be optimal set.

4.6.5 Computer Programme

On the basis of this LP model based on critical path concept a computer programme in FORTRAN IV language was prepared. This consists of one main program and one subroutine dia. This has been created on the disk of IIT/K DEC 1090 system, and has been found to work satisfactorily. The programme is however in the initial stage of development and needs full testing. The details are available with Dr. K. Subramanya.

5 CONCLUSIONS AND RECOMMENDATIONS

Five rural water supply schemes under U.P. Jal Nigam were selected for the present study. Three tree type schemes are in district Allahabad named Girdkot, Ketehra and Urwa and two loop cum tree type schemes are in district Kanpur named Malasa and Rajpur. The topography of the areas where these schemes fall are more or less flat. Overall comparative study of these schemes has been done. The need for an efficient design and analysis of water distribution networks has been brought out.

These schemes were analysed by an efficient modified water distribution network system analysis computer programme. By the analysis of these schemes, it was found that some schemes were under-designed and some over-designed. These deficiencies of the existing schemes were removed with the help of the programme. By making suitable changes in the pipe line data of the schemes, in few trials we have got nearby optimized networks and final cost was calculated for each scheme. On the basis of the comparative study of the schemes, it has been found that the per capita expenditure for a typical scheme, is Rs. 105/- for the year 2001.

A computer method for the optimum design of tree type water distribution network has been indicated. This

method is based on critical path approach.

It has been found that:

- i) There is scope for considerable improvement in the schemes to effect economy.
- ii) Computer analysis and design is a necessity and the traditional methods of design ~~have~~ to give way to this.
- iii) The analysis programme used in the present study is very fast, versatile, reliable and could be used by Jal Nigam and other organisations connected with water supply for obtaining status of their design/systems.
- iv) There is a need for a computer based optimal water supply design programme.

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APPENDIX I - FEATURES OF NETWORK ANALYSIS PROGRAMME

Features of the programme can be listed as follows:

1. Any type of pipe system configuration can be handled.
2. The system can contain any number of storage tanks, pumps, valves, meters, fittings etc.
3. Pumps can be described by useful power or by inputting head-flow data from operating curves. Out of range pump operation is incorporated into the programme.
4. The system can have pressure regulating valves which isolate entire low pressure regions or various single pressure regulations described throughout the system.
5. Check valves, which allow flow in only one direction, can be included.
6. Flow units of CFS, GPM, MGD or SI can be used.
7. Data preparation is simple even for large systems.
8. Complete output is provided including pressures, elevations and grade lines at all junctions, head losses in lines and at all valves, pump heads, flowrates and velocities.
9. The procedure is relatively fast. Typical computer times for execution on DEC 1090 system at IIT Kanpur are as follows:

For loop systems: 2-3 seconds for 100 pipes network

6-7 seconds for 300 pipes network

For tree type systems : 1-2 seconds for 100 pipes network.

10. It has the characteristics to converge to a solution for all situations.
11. No assumptions (such as initial flowrates or pressures) are required.
12. Sparse matrix routines are used which minimize storage requirements and increase computer execution times.
13. A pipe system of p pipes requires approximately $50 p$ dimensioned storage (word) spaces.

The basis of the programme is a direct solution of the basic pipe system hydraulic equations using a linearization scheme (A-1) and sparse matrix methods to handle the non-linearization methods to solve the equations. It utilizes linear network theory.

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APPENDIX II - RESULTS OF ANALYSIS

Typical output results relating to the analysis of all five U.P. Jal Nigam schemes (A through E) are appended.

SCHEME 1

GIRDKOT, ZONE A, GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, ALLAHABAD

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SCHEME HAS 112 PIPES, 112 JUNCTIONS, 0 LOOP AND 1 FIXED GRADE NODE

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1 112	30.00	20.00	130.00	21.805	0.083	0.694	2.760	44.49
2	1 112	700.00	20.00	130.00	12.015	0.641	0.382	2.915	
3	1 112	610.00	15.00	140.00	12.015	2.266	0.580	3.716	
4	1 112	433.00	15.00	140.00	14.198	1.441	0.535	3.328	
5	1 112	300.00	15.00	140.00	0.710	1.086	0.361	3.619	
6	1 112	230.00	15.00	140.00	1.750	0.440	0.348	3.952	
7	1 112	210.00	15.00	140.00	0.581	0.167	0.347	3.348	
8	1 112	210.00	15.00	140.00	0.097	0.453	0.322	3.155	
9	1 112	210.00	15.00	140.00	0.292	0.408	0.108	2.665	
10	1 112	27.00	15.00	140.00	1.383	1.548	0.154	2.687	
11	1 112	1255.00	15.00	140.00	0.583	0.642	0.275	2.361	
12	1 112	107.00	15.00	140.00	0.790	0.451	0.297	2.517	
13	1 112	95.00	15.00	140.00	2.786	0.096	0.407	2.509	
14	1 112	110.00	15.00	140.00	0.380	1.480	0.442	1.558	
15	1 112	110.00	15.00	140.00	0.380	0.458	0.309	1.522	
16	1 112	110.00	15.00	140.00	0.330	0.387	0.309	1.522	
17	1 112	300.00	15.00	140.00	1.548	0.830	0.371	3.796	
18	1 112	60.00	15.00	140.00	0.583	0.305	0.364	2.463	
19	1 112	80.00	15.00	140.00	1.167	0.203	0.352	2.522	
20	1 112	297.00	15.00	140.00	0.486	0.318	0.327	2.522	
21	1 112	478.00	15.00	140.00	0.486	0.479	0.341	2.805	
22	1 112	175.00	15.00	140.00	3.922	0.861	0.329	1.623	
23	1 112	175.00	15.00	140.00	0.194	2.463	0.396	1.623	
24	1 112	75.00	15.00	140.00	0.727	0.078	0.304	1.623	
25	1 112	90.00	15.00	140.00	0.194	0.866	0.350	1.623	
26	1 112	90.00	15.00	140.00	0.194	0.145	0.325	1.623	
27	1 112	500.00	15.00	140.00	0.226	0.156	0.277	1.623	
28	1 112	132.00	15.00	140.00	0.117	1.366	0.238	1.623	
29	1 112	132.00	15.00	140.00	0.262	0.435	0.222	1.623	
30	1 112	132.00	15.00	140.00	0.484	0.089	0.247	1.623	
31	1 112	132.00	15.00	140.00	0.484	0.178	0.247	1.623	
32	1 112	132.00	15.00	140.00	0.095	0.051	0.104	1.623	
33	1 112	132.00	15.00	140.00	0.095	0.389	0.104	1.623	
34	1 112	132.00	15.00	140.00	0.095	0.177	0.104	1.623	
35	1 112	132.00	15.00	140.00	0.194	0.211	0.242	1.623	
36	1 112	132.00	15.00	140.00	0.194	0.087	0.242	1.623	
37	1 112	132.00	15.00	140.00	0.194	0.015	0.242	1.623	
38	1 112	132.00	15.00	140.00	0.194	0.432	0.242	1.623	
39	1 112	132.00	15.00	140.00	0.194	0.451	0.242	1.623	
40	1 112	132.00	15.00	140.00	0.194	0.076	0.242	1.623	
41	1 112	132.00	15.00	140.00	0.194	0.241	0.242	1.623	
42	1 112	132.00	15.00	140.00	0.194	0.189	0.242	1.623	
43	1 112	132.00	15.00	140.00	0.194	0.073	0.242	1.623	
44	1 112	132.00	15.00	140.00	0.194	0.739	0.242	1.623	
45	1 112	132.00	15.00	140.00	0.194	0.451	0.242	1.623	
46	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
47	1 112	132.00	15.00	140.00	0.194	0.687	0.242	1.623	
48	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
49	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
50	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
51	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
52	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
53	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
54	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
55	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
56	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
57	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
58	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
59	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
60	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
61	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
62	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
63	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
64	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
65	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
66	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
67	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
68	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
69	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
70	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
71	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
72	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
73	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
74	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
75	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
76	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
77	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
78	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
79	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
80	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
81	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
82	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
83	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
84	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
85	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
86	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
87	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
88	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
89	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
90	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
91	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
92	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
93	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
94	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
95	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
96	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
97	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
98	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
99	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
100	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
101	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
102	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
103	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
104	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
105	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
106	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
107	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
108	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
109	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
110	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
111	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	
112	1 112	132.00	15.00	140.00	0.194	0.044	0.242	1.623	

[illegible]

JUNCTION NO.	DEMAND (GAL/SEC)	GRADE LINE (FEET)	ELEVATION (FEET)	PRESSURE (FEET)
1	0.00000	44.407	29.440	14.067
2	0.00000	41.500	29.250	12.253
3	0.00000	41.404	29.370	12.033
4	0.00000	40.544	30.740	9.015
5	0.00000	40.367	30.430	9.059
6	0.00000	39.013	31.420	7.499
7	0.00000	42.724	31.420	7.499
8	0.00000	41.733	29.440	13.189
9	0.00000	41.110	30.400	10.912
10	0.00000	40.704	30.400	10.912
11	0.00000	39.655	30.400	10.912
12	0.00000	39.191	30.500	8.779
13	0.35583	40.059	30.270	8.821
14	0.00000	40.973	30.530	8.821
15	0.38833	38.555	30.350	8.821
16	0.00000	33.953	30.220	8.821
17	0.00000	33.953	30.220	8.821
18	0.00000	33.953	30.220	8.821
19	0.00000	33.953	30.220	8.821
20	0.00000	33.953	30.220	8.821
21	0.00000	33.953	30.220	8.821
22	0.00000	33.953	30.220	8.821
23	0.00000	33.953	30.220	8.821
24	0.00000	33.953	30.220	8.821
25	0.00000	33.953	30.220	8.821
26	0.00000	33.953	30.220	8.821
27	0.00000	33.953	30.220	8.821
28	0.00000	33.953	30.220	8.821
29	0.00000	33.953	30.220	8.821
30	0.00000	33.953	30.220	8.821
31	0.00000	33.953	30.220	8.821
32	0.00000	33.953	30.220	8.821
33	0.00000	33.953	30.220	8.821
34	0.00000	33.953	30.220	8.821
35	0.00000	33.953	30.220	8.821
36	0.00000	33.953	30.220	8.821
37	0.00000	33.953	30.220	8.821
38	0.00000	33.953	30.220	8.821
39	0.00000	33.953	30.220	8.821
40	0.00000	33.953	30.220	8.821
41	0.00000	33.953	30.220	8.821
42	0.00000	33.953	30.220	8.821
43	0.00000	33.953	30.220	8.821
44	0.00000	33.953	30.220	8.821
45	0.00000	33.953	30.220	8.821
46	0.00000	33.953	30.220	8.821
47	0.00000	33.953	30.220	8.821
48	0.00000	33.953	30.220	8.821
49	0.00000	33.953	30.220	8.821
50	0.00000	33.953	30.220	8.821
51	0.00000	33.953	30.220	8.821
52	0.00000	33.953	30.220	8.821
53	0.00000	33.953	30.220	8.821
54	0.00000	33.953	30.220	8.821
55	0.00000	33.953	30.220	8.821
56	0.00000	33.953	30.220	8.821
57	0.00000	33.953	30.220	8.821
58	0.00000	33.953	30.220	8.821
59	0.00000	33.953	30.220	8.821
60	0.00000	33.953	30.220	8.821
61	0.00000	33.953	30.220	8.821
62	0.00000	33.953	30.220	8.821
63	0.00000	33.953	30.220	8.821
64	0.00000	33.953	30.220	8.821
65	0.00000	33.953	30.220	8.821
66	0.00000	33.953	30.220	8.821
67	0.00000	33.953	30.220	8.821
68	0.00000	33.953	30.220	8.821
69	0.00000	33.953	30.220	8.821
70	0.00000	33.953	30.220	8.821
71	0.00000	33.953	30.220	8.821
72	0.00000	33.953	30.	

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEME

DIAMETER (CM'S)	ROUGHNESS	RATE (/RS)	TOTAL LENGTH (METERS)	COST (RS)
25.00	130.00	55.00	0.00	0.00
25.00	130.00	40.00	1910.00	76400.00
15.00	130.00	30.00	1067.00	32010.00
15.00	130.00	25.00	1131.00	28275.00
10.00	140.00	17.80	5021.00	89373.80
10.00	140.00	12.70	3892.00	49428.40

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THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	MODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS
28	5	250.0	140.0	140.0
29	31	312.0	140.0	140.0
31	33	162.0	140.0	140.0
32	34	300.0	140.0	140.0
33	35	20.0	140.0	140.0
34	36	150.0	140.0	140.0
35	37	20.0	140.0	140.0
36	38	20.0	140.0	140.0
37	39	20.0	140.0	140.0
38	40	20.0	140.0	140.0
39	41	20.0	140.0	140.0
40	42	20.0	140.0	140.0
41	43	20.0	140.0	140.0
42	44	20.0	140.0	140.0
43	45	20.0	140.0	140.0
44	46	20.0	140.0	140.0
45	47	20.0	140.0	140.0
46	48	20.0	140.0	140.0
47	49	20.0	140.0	140.0
48	50	20.0	140.0	140.0
49	51	20.0	140.0	140.0
50	52	20.0	140.0	140.0
51	53	20.0	140.0	140.0
52	54	20.0	140.0	140.0
53	55	20.0	140.0	140.0
54	56	20.0	140.0	140.0
55	57	20.0	140.0	140.0
56	58	20.0	140.0	140.0
57	59	20.0	140.0	140.0
58	60	20.0	140.0	140.0
59	61	20.0	140.0	140.0
60	62	20.0	140.0	140.0
61	63	20.0	140.0	140.0
62	64	20.0	140.0	140.0
63	65	20.0	140.0	140.0
64	66	20.0	140.0	140.0
65	67	20.0	140.0	140.0
66	68	20.0	140.0	140.0
67	69	20.0	140.0	140.0
68	70	20.0	140.0	140.0
69	71	20.0	140.0	140.0
70	72	20.0	140.0	140.0
71	73	20.0	140.0	140.0
72	74	20.0	140.0	140.0
73	75	20.0	140.0	140.0
74	76	20.0	140.0	140.0
75	77	20.0	140.0	140.0
76	78	20.0	140.0	140.0
77	79	20.0	140.0	140.0
78	80	20.0	140.0	140.0
79	81	20.0	140.0	140.0
80	82	20.0	140.0	140.0
81	83	20.0	140.0	140.0
82	84	20.0	140.0	140.0
83	85	20.0	140.0	140.0
84	86	20.0	140.0	140.0
85	87	20.0	140.0	140.0
86	88	20.0	140.0	140.0
87	89	20.0	140.0	140.0
88	90	20.0	140.0	140.0
89	91	20.0	140.0	140.0
90	92	20.0	140.0	140.0
91	93	20.0	140.0	140.0
92	94	20.0	140.0	140.0
93	95	20.0	140.0	140.0
94	96	20.0	140.0	140.0
95	97	20.0	140.0	140.0
96	98	20.0	140.0	140.0
97	99	20.0	140.0	140.0
98	100	20.0	140.0	140.0
99	101	20.0	140.0	140.0
100	102	20.0	140.0	140.0
101	103	20.0	140.0	140.0
102	104	20.0	140.0	140.0
103	105	20.0	140.0	140.0
104	106	20.0	140.0	140.0
105	107	20.0	140.0	140.0
106	108	20.0	140.0	140.0
107	109	20.0	140.0	140.0
108	110	20.0	140.0	140.0
109	111	20.0	140.0	140.0
110	112	20.0	140.0	140.0
111	113	20.0	140.0	140.0
112	114	20.0	140.0	140.0
113	115	20.0	140.0	140.0
114	116	20.0	140.0	140.0
115	117	20.0	140.0	140.0
116	118	20.0	140.0	140.0
117	119	20.0	140.0	140.0
118	120	20.0	140.0	140.0
119	121	20.0	140.0	140.0
120	122	20.0	140.0	140.0
121	123	20.0	140.0	140.0
122	124	20.0	140.0	140.0
123	125	20.0	140.0	140.0
124	126	20.0	140.0	140.0
125	127	20.0	140.0	140.0
126	128	20.0	140.0	140.0
127	129	20.0	140.0	140.0
128	130	20.0	140.0	140.0
129	131	20.0	140.0	140.0
130	132	20.0	140.0	140.0
131	133	20.0	140.0	140.0
132	134	20.0	140.0	140.0
133	135	20.0	140.0	140.0
134	136	20.0	140.0	140.0
135	137	20.0	140.0	140.0
136	138	20.0	140.0	140.0
137	139	20.0	140.0	140.0
138	140	20.0	140.0	140.0
139	141	20.0	140.0	140.0
140	142	20.0	140.0	140.0
141	143	20.0	140.0	140.0
142	144	20.0	140.0	140.0
143	145	20.0	140.0	140.0
144	146	20.0	140.0	140.0
145	147	20.0	140.0	140.0
146	148	20.0	140.0	140.0
147	149	20.0	140.0	140.0
148	150	20.0	140.0	140.0
149	151	20.0	140.0	140.0
150	152	20.0	140.0	140.0
151	153	20.0	140.0	140.0
152	154	20.0	140.0	140.0
153	155	20.0	140.0	140.0
154	156	20.0	140.0	140.0
155	157	20.0	140.0	140.0
156	158	20.0	140.0	140.0
157	159	20.0	140.0	140.0
158	160	20.0	140.0	140.0
159	161	20.0	140.0	140.0
160	162	20.0	140.0	140.0
161	163	20.0	140.0	140.0
162	164	20.0	140.0	140.0
163	165	20.0	140.0	140.0
164	166	20.0	140.0	140.0
165	167	20.0	140.0	140.0
166	168	20.0	140.0	140.0
167	169	20.0	140.0	140.0
168	170	20.0	140.0	140.0
169	171	20.0	140.0	140.0
170	172	20.0	140.0	140.0
171	173	20.0	140.0	140.0
172	174	20.0	140.0	140.0
173	175	20.0	140.0	140.0
174	176	20.0	140.0	140.0
175	177	20.0	140.0	140.0
176	178	20.0	140.0	140.0
177	179	20.0	140.0	140.0
178	180	20.0	140.0	140.0
179	181	20.0	140.0	140.0
180	182	20.0	140.0	140.0
181	183	20.0	140.0	140.0
182	184	20.0	140.0	140.0
183	185	20.0	140.0	140.0
184	186	20.0	140.0	140.0
185	187	20.0	140.0	140.0
186	188	20.0	140.0	140.0
187	189	20.0	140.0	140.0
188	190	20.0	140.0	140.0
189	191	20.0	140.0	140.0
190	192	20.0	140.0	140.0
191	193	20.0	140.0	140.0
192	194	20.0	140.0	140.0
193	195	20.0	140.0	140.0
194	196	20.0	140.0	140.0
195	197	20.0	140.0	140.0
196	198	20.0	140.0	140.0
197	199	20.0	140.0	140.0
198	200	20.0	140.0	140.0
199	201	20.0	140.0	140.0
200	202	20.0	140.0	140.0
201	203	20.0	140.0	140.0
202	204	20.0	140.0	140.0
203	205	20.0	140.0	140.0
204	206	20.0	140.0	140.0
205	207	20.0	140.0	140.0
206	208	20.0	140.0	140.0
207	209	20.0	140.0	140.0
208	210	20.0	140.0	140.0
209	211	20.0	140.0	140.0
210	212	20.0	140.0	140.0
211	213	20.0	140.0	140.0
212	214	20.0	140.0	140.0
213	215	20.0	140.0	140.0
214	216	20.0	140.0	140.0
215	217	20.0	140.0	140.0
216	218	20.0	140.0	140.0
217	219	20.0	140.0	140.0
218	220	20.0	140.0	140.0
219	221	20.0	140.0	140.0
220	222	20.0	140.0	140.0
221	223	20.0	140.0	140.0
222	224	20.0	140.0	140.0
223	225	20.0	140.0	140.0
224	226	20.0	140.0	140.0
225	227	20.0	140.0	140.0
226	228	20.0	140.0	140.0
227	229	20.0	140.0	140.0
228	230	20.0	140.0	140.0
229	231	20.0	140.0	140.0
230	232	20.0	140.0	140.0
231	233	20.0	140.0	140.0
232	234	20.0	140.0	140.0
233	235	20.0	140.0	140.0
234	236	20.0	140.0	140.0
235	237	20.0	140.0	140.0
236	238	20.0	140.0	140.0
237	239	20.0	140.0	140.0
238	240	20.0	140.0	140.0
239	241	20.0	140.0	140.0
240	242	20.0	140.0	140.0
241	243	20.0	140.0	140.0
242	244	20.0	140.0	140.0
243	245	20.0	140.0	140.0
244	246	20.0	140.0	140.0
245	247	20.0	140.0	140.0
246	248	20.0	140.0	140.0
247	249	20.0	140.0	140.0
248	250	20.0	140.0	140.0
249	251	20.0	140.0	140.0
250	252	20.0	140.0	140.0
251	253	20.0	140.0	140.0
252	254	20.0	140.0	140.0
253	255	20.0	140.0	140.0
254	256	20.0	140.0	140.0
255	257	20.0	140.0	140.0
256	258	20.0	140.0	140.0
257	259	20.0	140.0	140.0
258	260	20.0	140.0	140.0
259	261	20.0	140.0	140.0
260	262	20.0	140.0	140.0
261	263	20.0	140.0	140.0
262	264	20.0	140.0	140.0
263	265	20.0	140.0	140.0
264	266	20.0	140.0	140.0
265	267	20.0	140.0	140.0
266	268	20.0	140.0	140.0
267	269	20.0	140.0	140.0
268	270	20.0	140.0	140.0
269	271	20.0	140.0	140.0
270	272	20.0	140.0	140.0
271	273	20.0	140.0	140.0
272	274	20.0	140.0	140.0
273	275	20.0	140.0	140.0
274	276	20.0	140.0	140.0
275	277	20.0	140.0	140.0
276	278	20.0	140.0	140.0

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

INJECTION NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.00000	44.407	29.440	14.207
2	0.00000	41.500	29.250	12.033
3	0.00000	41.404	29.370	11.000
4	0.00000	40.544	30.710	9.999
5	0.00000	40.347	31.420	7.749
6	0.00000	39.913	31.440	7.289
7	0.00000	39.724	30.440	6.122
8	0.00000	41.633	30.300	5.269
9	0.00000	41.110	30.180	4.669
10	0.00000	40.701	30.100	4.274
11	0.00000	39.635	30.600	3.669
12	0.00000	40.052	30.250	3.744
13	0.00000	38.997	30.250	3.669
14	0.00000	38.997	30.250	3.669
15	0.00000	38.997	30.250	3.669

105	0.00000	38.489	30.890	7.60
106	0.05833	38.385	30.890	7.50
107	0.00000	38.077	30.700	7.38
108	0.17500	37.246	30.500	6.75
109	0.38889	37.725	30.600	7.03
110	0.05056	38.033	30.700	7.33
111	0.01944	37.986	30.690	7.30
112	0.00000	43.766	29.250	14.52

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEMP

DIAMETER (CMS)	ROUGHNESS	RATE (/M(RS)	TOTAL LENGTH (METERS)	COST (RS)
25.00	130.0	55.00	0.0	0.00
20.00	130.0	40.00	1919.0	76400.00
15.00	130.0	30.00	1067.0	32010.00
12.00	130.0	25.00	1131.0	28275.00
10.00	140.0	17.80	5241.0	93289.80
8.00	140.0	12.70	3372.0	20444.40
5.00	140.0	9.70	3180.0	24056.00
3.25	140.0	7.00	1172.0	8204.00
	140.0	4.70	1430.0	6721.00
	140.0	4.15	2195.0	9109.25
	140.0	4.10	3377.0	13845.70
TOTAL COST=				342355.15

THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS
89	107	105.0	3.2	140.0
71	73	300.0	4.0	140.0
66	71	200.0	3.2	140.0
62	68	200.0	3.2	140.0
68	83	78.0	4.0	140.0
103	88	100.0	4.0	140.0
22	61	90.0	3.2	140.0
21	59	90.0	3.2	140.0
18	56	90.0	3.2	140.0
12	53	255.0	5.0	140.0

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

TYPE NO.	MODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLUOR RATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1	30	0	130	21.805	0.083	0.000	20	760
1	1	700	0	130	12.015	0.041	0.000	20	015
1	1	613	0	140	12.015	0.041	0.000	20	715
1	1	300	0	140	4.710	0.041	0.000	20	320
1	1	500	0	140	0.710	0.041	0.000	20	610
1	1	500	0	140	0.710	0.041	0.000	20	920
1	1	500	0	140	0.710	0.041	0.000	20	520
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500	0	140	0.710	0.041	0.000	20	168
1	1	500	0	140	0.710	0.041	0.000	20	366
1	1	500	0	140	0.710	0.041	0.000	20	155
1	1	500	0	140	0.710	0.041	0.000	20	387
1	1	500							

741	1	742	1	743	1	744	1	745	1	746	1	747	1	748	1	749	1	750	1	751	1	752	1	753	1	754	1	755	1	756	1	757	1	758	1	759	1	760	1	761	1	762	1	763	1	764	1	765	1	766	1	767	1	768	1	769	1	770	1	771	1	772	1	773	1	774	1	775	1	776	1	777	1	778	1	779	1	780	1	781	1	782	1	783	1	784	1	785	1	786	1	787	1	788	1	789	1	790	1	791	1	792	1	793	1	794	1	795	1	796	1	797	1	798	1	799	1	800	1	801	1	802	1	803	1	804	1	805	1	806	1	807	1	808	1	809	1	810	1	811	1	812	1	813	1	814	1	815	1	816	1	817	1	818	1	819	1	820	1	821	1	822	1	823	1	824	1	825	1	826	1	827	1	828	1	829	1	830	1	831	1	832	1	833	1	834	1	835	1	836	1	837	1	838	1	839	1	840	1	841	1	842	1	843	1	844	1	845	1	846	1	847	1	848	1	849	1	850	1	851	1	852	1	853	1	854	1	855	1	856	1	857	1	858	1	859	1	860	1	861	1	862	1	863	1	864	1	865	1	866	1	867	1	868	1	869	1	870	1	871	1	872	1	873	1	874	1	875	1	876	1	877	1	878	1	879	1	880	1	881	1	882	1	883	1	884	1	885	1	886	1	887	1	888	1	889	1	890	1	891	1	892	1	893	1	894	1	895	1	896	1	897	1	898	1	899	1	900	1	901	1	902	1	903	1	904	1	905	1	906	1	907	1	908	1	909	1	910	1	911	1	912	1	913	1	914	1	915	1	916	1	917	1	918	1	919	1	920	1	921	1	922	1	923	1	924	1	925	1	926	1	927	1	928	1	929	1	930	1	931	1	932	1	933	1	934	1	935	1	936	1	937	1	938	1	939	1	940	1	941	1	942	1	943	1	944	1	945	1	946	1	947	1	948	1	949	1	950	1	951	1	952	1	953	1	954	1	955	1	956	1	957	1	958	1	959	1	960	1	961	1	962	1	963	1	964	1	965	1	966	1	967	1	968	1	969	1	970	1	971	1	972	1	973	1	974	1	975	1	976	1	977	1	978	1	979	1	980	1	981	1	982	1	983	1	984	1	985	1	986	1	987	1	988	1	989	1	990	1	991	1	992	1	993	1	994	1	995	1	996	1	997	1	998	1	999	1	1000	1	1001	1	1002	1	1003	1	1004	1	1005	1	1006	1	1007	1	1008	1	1009	1	1010	1	1011	1	1012	1	1013	1	1014	1	1015	1	1016	1	1017	1	1018	1	1019	1	1020	1	1021	1	1022	1	1023	1	1024	1	1025	1	1026	1	1027	1	1028	1	1029	1	1030	
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11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100 101 102 103 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119 120 121 122 123 124 125 126 127 128 129 130 131 132 133 134 135 136 137 138 139 140 141 142 143 144 145 146 147 148 149 150 151 152 153 154 155 156 157 158 159 160 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182 183 184 185 186 187 188 189 190 191 192 193 194 195 196 197 198 199 200 201 202 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 219 220 221 222 223 224 225 226 227 228 229 230 231 232 233 234 235 236 237 238 239 240 241 242 243 244 245 246 247 248 249 250 251 252 253 254 255 256 257 258 259 260 261 262 263 264 265 266 267 268 269 270 271 272 273 274 275 276 277 278 279 280 281 282 283 284 285 286 287 288 289 290 291 292 293 294 295 296 297 298 299 300 301 302 303 304 305 306 307 308 309 310 311 312 313 314 315 316 317 318 319 320 321 322 323 324 325 326 327 328 329 330 331 332 333 334 335 336 337 338 339 340 341 342 343 344 345 346 347 348 349 350 351 352 353 354 355 356 357 358 359 360 361 362 363 364 365 366 367 368 369 370 371 372 373 374 375 376 377 378 379 380 381 382 383 384 385 386 387 388 389 390 391 392 393 394 395 396 397 398 399 400 401 402 403 404 405 406 407 408 409 410 411 412 413 414 415 416 417 418 419 420 421 422 423 424 425 426 427 428 429 430 431 432 433 434 435 436 437 438 439 440 441 442 443 444 445 446 447 448 449 450 451 452 453 454 455 456 457 458 459 460 461 462 463 464 465 466 467 468 469 470 471 472 473 474 475 476 477 478 479 480 481 482 483 484 485 486 487 488 489 490 491 492 493 494 495 496 497 498 499 500 501 502 503 504 505 506 507 508 509 510 511 512 513 514 515 516 517 518 519 520 521 522 523 524 525 526 527 528 529 530 531 532 533 534 535 536 537 538 539 540 541 542 543 544 545 546 547 548 549 550 551 552 553 554 555 556 557 558 559 560 561 562 563 564 565 566 567 568 569 570 571 572 573 574 575 576 577 578 579 580 581 582 583 584 585 586 587 588 589 590 591 592 593 594 595 596 597 598 599 600 601 602 603 604 605 606 607 608 609 610 611 612 613 614 615 616 617 618 619 620 621 622 623 624 625 626 627 628 629 630 631 632 633 634 635 636 637 638 639 640 641 642 643 644 645 646 647 648 649 650 651 652 653 654 655 656 657 658 659 660 661 662 663 664 665 666 667 668 669 670 671 672 673 674 675 676 677 678 679 680 681 682 683 684 685 686 687 688 689 690 691 692 693 694 695 696 697 698 699 700 701 702 703 704 705 706 707 708 709 710 711 712 713 714 715 716 717 718 719 720 721 722 723 724 725 726 727 728 729 730 731 732 733 734 735 736 737 738 739 740 741 742 743 744 745 746 747 748 749 750 751 752 753 754 755 756 757 758 759 760 761 762 763 764 765 766 767 768 769 770 771 772 773 774 775 776 777 778 779 780 781 782 783 784 785 786 787 788 789 790 791 792 793 794 795 796 797 798 799 800 801 802 803 804 805 806 807 808 809 810 811 812 813 814 815 816 817 818 819 820 821 822 823 824 825 826 827 828 829 830 831 832 833 834 835 836 837 838 839 840 841 842 843 844 845 846 847 848 849 850 851 852 853 854 855 856 857 858 859 860 861 862 863 864 865 866 867 868 869 870 871 872 873 874 875 876 877 878 879 880 881 882 883 884 885 886 887 888 889 890 891 892 893 894 895 896 897 898 899 900 901 902 903 904 905 906 907 908 909 910 911 912 913 914 915 916 917 918 919 920 921 922 923 924 925 926 927 928 929 930 931 932 933 934 935 936 937 938 939 940 941 942 943 944 945 946 947 948 949 950 951 952 953 954 955 956 957 958 959 960 961 962 963 964 965 966 967 968 969 970 971 972 973 974 975 976 977 978 979 980 981 982 983 984 985 986 987 988 989 990 991 992 993 994 995 996 997 998 999 1000 1001 1002 1003 1004 1005 1006 1007 1008 1009 1010 1011 1012 1013 1014 1015 1016 1017 1018 1019 1020 1021 1022 1023 1024 1025 1026 1027 1028 1029 1030 1031 1032 1033 1034 1035 1036 1037 1038 1039 1040 1041 1042 1043 1044 10

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THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

SECTION NO.	DEMAND (LIT/SEC)	GRADE LINE (FEET/RS)	ELEVATION (FEET/RS)	PRESSURE (FEET/RS)
1	0.000000	44.4007	29.440	14.207
2	0.000000	41.504	29.257	12.033
3	0.000000	41.544	30.270	12.033
4	0.000000	40.542	30.430	12.033
5	0.000000	38.723	30.440	12.033
6	0.000000	41.633	30.440	12.033
7	0.000000	41.110	30.440	12.033
8	0.000000	40.701	30.180	12.033
9	0.000000	39.655	30.180	12.033
10	0.000000	39.191	30.180	12.033
11	0.000000	38.033	30.230	12.033
12	0.000000	38.554	30.230	12.033
13	0.000000	37.551	30.230	12.033
14	0.000000	37.610	30.230	12.033
15	0.000000	36.137	30.230	12.033
16	0.000000	36.000	30.230	12.033
17	0.000000	35.000	30.230	12.033
18	0.000000	34.000	30.230	12.033
19	0.000000	33.000	30.230	12.033
20	0.000000	32.000	30.230	12.033
21	0.000000	31.000	30.230	12.033
22	0.000000	30.000	30.230	12.033
23	0.000000	29.000	30.230	12.033
24	0.000000	28.000	30.230	12.033
25	0.000000	27.000	30.230	12.033
26	0.000000	26.000	30.230	12.033
27	0.000000	25.000	30.230	12.033
28	0.000000	24.000	30.230	12.033
29	0.000000	23.000	30.230	12.033
30	0.000000	22.000	30.230	12.033
31	0.000000	21.000	30.230	12.033
32	0.000000	20.000	30.230	12.033
33	0.000000	19.000	30.230	12.033
34	0.000000	18.000	30.230	12.033
35	0.000000	17.000	30.230	12.033
36	0.000000	16.000	30.230	12.033
37	0.000000	15.000	30.230	12.033
38	0.000000	14.000	30.230	12.033
39	0.000000	13.000	30.230	12.033
40	0.000000	12.000	30.230	12.033
41	0.000000	11.000	30.230	12.033
42	0.000000	10.000	30.230	12.033
43	0.000000	9.000	30.230	12.033
44	0.000000	8.000	30.230	12.033
45	0.000000	7.000	30.230	12.033
46	0.000000	6.000	30.230	12.033
47	0.000000	5.000	30.230	12.033
48	0.000000	4.000	30.230	12.033
49	0.000000	3.000	30.230	12.033
50	0.000000	2.000	30.230	12.033
51	0.000000	1.000	30.230	12.033
52	0.000000	0.000	30.230	12.033

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SCHEME 2

KETEHRRA, ZONE A, GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, ALLAHABAD

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SCHEME HAS 55 PIPES, 55 JUNCTIONS, 0 LOOP AND 1 FIXED GRADE NODE

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	MODE NO.	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1	10.00	15.00	130.00	20.228	0.097	1.145	9.750	117.20
2	2	30.00	15.00	130.00	14.343	0.155	0.812	5.158	
3	3	3880.00	10.00	140.00	4.117	1.531	0.552	4.802	
4	4	3000.00	10.00	140.00	4.644	3.337	0.591	4.013	
5	5	3000.00	10.00	140.00	1.513	2.718	0.459	4.090	
6	6	3000.00	10.00	140.00	0.284	2.328	0.289	4.322	
7	7	3000.00	10.00	140.00	0.846	2.486	0.362	4.621	
8	8	1300.00	10.00	140.00	0.378	0.128	0.120	1.621	
9	9	1300.00	10.00	140.00	0.226	2.645	0.470	2.211	
10	10	1300.00	10.00	140.00	0.295	1.463	0.529	2.119	
11	11	1300.00	10.00	140.00	1.284	1.463	0.529	2.278	
12	12	1300.00	10.00	140.00	1.496	1.534	0.549	2.516	
13	13	1300.00	10.00	140.00	1.770	1.534	0.549	2.709	
14	14	1300.00	10.00	140.00	3.119	1.534	0.549	3.374	
15	15	1300.00	10.00	140.00	5.027	1.229	0.343	3.527	
16	16	1300.00	10.00	140.00	4.027	1.229	0.343	3.082	
17	17	1300.00	10.00	140.00	1.899	1.229	0.343	2.597	
18	18	1300.00	10.00	140.00	0.692	1.229	0.343	2.100	
19	19	1300.00	10.00	140.00	0.892	1.229	0.343	2.133	
20	20	1300.00	10.00	140.00	2.420	1.229	0.343	2.660	
21	21	1300.00	10.00	140.00	2.950	1.229	0.343	3.120	
22	22	1300.00	10.00	140.00	1.684	1.229	0.343	2.507	
23	23	1300.00	10.00	140.00	2.284	1.229	0.343	2.864	
24	24	1300.00	10.00	140.00	2.284	1.229	0.343	2.864	
25	25	1300.00	10.00	140.00	1.388	1.229	0.343	2.503	
26	26	1300.00	10.00	140.00	0.416	1.229	0.343	2.178	
27	27	1300.00	10.00	140.00	1.114	1.229	0.343	2.370	
28	28	1300.00	10.00	140.00	1.885	1.229	0.343	2.657	
29	29	1300.00	10.00	140.00	2.284	1.229	0.343	2.864	
30	30	1300.00	10.00	140.00	2.284	1.229	0.343	2.864	
31	31	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
32	32	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
33	33	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
34	34	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
35	35	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
36	36	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
37	37	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
38	38	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
39	39	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
40	40	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
41	41	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
42	42	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
43	43	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
44	44	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
45	45	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
46	46	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
47	47	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
48	48	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
49	49	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
50	50	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
51	51	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
52	52	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
53	53	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
54	54	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	
55	55	1300.00	10.00	140.00	1.805	1.229	0.343	2.500	

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

JUNCTION NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.00000	117.103	100.030	17.07
2	0.00000	117.103	99.620	17.33
3	0.00000	115.267	99.310	15.96
4	0.00000	111.736	99.790	11.95
5	0.94583	109.399	99.530	9.87
6	1.90112	111.249	99.910	11.34

[illegible]

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEME

DIAMETER (CM)	ROUGHNESS	RATE (/H(RS))	TOTAL LENGTH (METERS)	COST (RS)
130.00	130.00	55.00	0.00	0.00
130.00	130.00	40.00	0.00	0.00
130.00	130.00	35.00	160.00	4920.00
130.00	130.00	25.00	1780.00	4425.00
130.00	130.00	12.00	5725.00	69103.50
140.00	140.00	7.00	1840.00	12769.00
140.00	140.00	4.15	2750.00	12880.00
140.00	140.00	4.10	1230.00	5104.50
140.00	140.00		2140.00	8774.00
		TOTAL COST=		31233.00

THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS
6	5	300.0	4.0	140.0
55	3	400.0	2.5	140.0
31	32	300.0	3.2	140.0
28	30	200.0	2.5	140.0
30	32	210.0	3.2	140.0

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1	10.0	15.0	130.0	14.0	0.097	1.145	9.750	117.20
2	2	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
3	3	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
4	4	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
5	5	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
6	6	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
7	7	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
8	8	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
9	9	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
10	10	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
11	11	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
12	12	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
13	13	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
14	14	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
15	15	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
16	16	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
17	17	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
18	18	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
19	19	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
20	20	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
21	21	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
22	22	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
23	23	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
24	24	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
25	25	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
26	26	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
27	27	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
28	28	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
29	29	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
30	30	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
31	31	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
32	32	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
33	33	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
34	34	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
35	35	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
36	36	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
37	37	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
38	38	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
39	39	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
40	40	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
41	41	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
42	42	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
43	43	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
44	44	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
45	45	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
46	46	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
47	47	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
48	48	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
49	49	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
50	50	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
51	51	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
52	52	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
53	53	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
54	54	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
55	55	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
56	56	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
57	57	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
58	58	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
59	59	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
60	60	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
61	61	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
62	62	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
63	63	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
64	64	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
65	65	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
66	66	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
67	67	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
68	68	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
69	69	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
70	70	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
71	71	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
72	72	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
73	73	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
74	74	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
75	75	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
76	76	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
77	77	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
78	78	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
79	79	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
80	80	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
81	81	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
82	82	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
83	83	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
84	84	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
85	85	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
86	86	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
87	87	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
88	88	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
89	89	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
90	90	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
91	91	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
92	92	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
93	93	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
94	94	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
95	95	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
96	96	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
97	97	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
98	98	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
99	99	350.0	10.0	140.0	15.0	1.681	0.852	5.158	
100	100	350.0	10.0	140.0	15.0	1.681	0.852	5.158	

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

JUNCTION NO.	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	117.103	100.030	17.073
2	116.948	99.620	17.328
3	115.267	99.310	15.957
4	111.736	99.790	11.946
5	109.390	99.530	9.860
6	111.240	99.900	11.340
7	111.123	99.890	11.233
8	108.812	99.890	8.922

SCHEME 3

URWA, PART II, GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, ALLAHABAD

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SCHEME HAS 138 PIPES, 138 JUNCTIONS, 0 LOOP AND 1 FIXED GRADE NODE

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1-2	100.0	25.0	130.0	61.912	0.643	1.261	6.432	114.00
2	2-3	340.0	25.0	130.0	26.526	1.004	0.540	8.478	114.00
3	3-4	140.0	20.0	130.0	16.531	0.231	0.526	1.653	114.00
4	4-5	150.0	20.0	130.0	15.402	2.305	0.400	1.450	114.00
5	5-6	900.0	20.0	130.0	15.125	0.424	0.455	4.245	114.00
6	6-7	200.0	4.2	130.0	0.160	0.282	0.483	1.248	114.00
7	7-8	450.0	20.0	130.0	0.375	4.351	0.398	1.508	114.00
8	8-9	615.0	20.0	130.0	14.410	0.788	0.459	5.732	114.00
9	9-10	550.0	20.0	130.0	12.972	0.403	0.455	1.608	114.00
10	10-11	140.0	25.0	140.0	1.389	0.432	0.509	1.732	114.00
11	11-12	140.0	4.0	140.0	0.389	0.432	0.355	3.021	114.00
12	12-13	140.0	0.5	140.0	1.325	0.352	0.355	3.021	114.00
13	13-14	140.0	0.5	140.0	0.422	1.047	0.615	1.360	114.00
14	14-15	140.0	0.5	140.0	0.712	1.047	0.615	1.360	114.00
15	15-16	140.0	0.5	140.0	0.475	1.628	0.454	1.372	114.00
16	16-17	140.0	0.5	140.0	0.853	1.628	0.604	1.770	114.00
17	17-18	140.0	0.5	140.0	0.243	0.391	0.302	15.813	114.00
18	18-19	140.0	0.5	140.0	0.638	0.391	0.432	15.813	114.00
19	19-20	140.0	0.5	140.0	0.194	1.323	0.326	14.550	114.00
20	20-21	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
21	21-22	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
22	22-23	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
23	23-24	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
24	24-25	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
25	25-26	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
26	26-27	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
27	27-28	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
28	28-29	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
29	29-30	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
30	30-31	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
31	31-32	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
32	32-33	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
33	33-34	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
34	34-35	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
35	35-36	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
36	36-37	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
37	37-38	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
38	38-39	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
39	39-40	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
40	40-41	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
41	41-42	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
42	42-43	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
43	43-44	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
44	44-45	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
45	45-46	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
46	46-47	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
47	47-48	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
48	48-49	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
49	49-50	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
50	50-51	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
51	51-52	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
52	52-53	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
53	53-54	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
54	54-55	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
55	55-56	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
56	56-57	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
57	57-58	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
58	58-59	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
59	59-60	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
60	60-61	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
61	61-62	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
62	62-63	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
63	63-64	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
64	64-65	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
65	65-66	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
66	66-67	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
67	67-68	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
68	68-69	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
69	69-70	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
70	70-71	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
71	71-72	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
72	72-73	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
73	73-74	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
74	74-75	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
75	75-76	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
76	76-77	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
77	77-78	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
78	78-79	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
79	79-80	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
80	80-81	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
81	81-82	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
82	82-83	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
83	83-84	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
84	84-85	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
85	85-86	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
86	86-87	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
87	87-88	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
88	88-89	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
89	89-90	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
90	90-91	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
91	91-92	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
92	92-93	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
93	93-94	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
94	94-95	140.0	0.5	140.0	0.222	1.323	0.326	14.550	114.00
95	95-96	140.0							

[illegible]

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

JUNCTION NO.	DEMAND (CFS/SEC.)	GRADE LINE (FEET)	ELEVATION (FEET)	PRESSURE (FEET)
1	0.0000	113.357	100.200	13.16
2	0.6250	110.474	99.870	10.60
3	0.0000	112.353	100.410	11.94
4	0.0000	109.999	100.180	9.82
5	0.0000	112.122	100.240	11.88
6	0.4861	109.755	99.870	9.88
7	0.1167	110.817	99.400	11.39
8	0.1250	110.392	99.400	10.99
9	0.2500	110.535	99.100	11.44
10	0.1250	110.743	98.830	11.91
11	0.3750	105.751	98.760	16.99
12	0.5680	103.747	98.600	15.15

102	25670	106	540	96	570	88	7
103	20000	107	547	106	750	77	7
104	13890	108	541	107	600	77	7
105	13890	109	542	108	460	77	7
106	13890	110	543	109	350	77	7
107	13890	111	544	110	300	77	7
108	13890	112	545	111	300	77	7
109	13890	113	546	112	300	77	7
110	13890	114	547	113	300	77	7
111	13890	115	548	114	300	77	7
112	13890	116	549	115	300	77	7
113	13890	117	550	116	300	77	7
114	13890	118	551	117	300	77	7
115	13890	119	552	118	300	77	7
116	13890	120	553	119	300	77	7
117	13890	121	554	120	300	77	7
118	13890	122	555	121	300	77	7
119	13890	123	556	122	300	77	7
120	13890	124	557	123	300	77	7
121	13890	125	558	124	300	77	7
122	13890	126	559	125	300	77	7
123	13890	127	560	126	300	77	7
124	13890	128	561	127	300	77	7
125	13890	129	562	128	300	77	7
126	13890	130	563	129	300	77	7
127	13890	131	564	130	300	77	7
128	13890	132	565	131	300	77	7
129	13890	133	566	132	300	77	7
130	13890	134	567	133	300	77	7
131	13890	135	568	134	300	77	7
132	13890	136	569	135	300	77	7
133	13890	137	570	136	300	77	7
134	13890	138	571	137	300	77	7
135	13890	139	572	138	300	77	7
136	13890	140	573	139	300	77	7
137	13890	141	574	140	300	77	7
138	13890	142	575	141	300	77	7
139	13890	143	576	142	300	77	7
140	13890	144	577	143	300	77	7
141	13890	145	578	144	300	77	7
142	13890	146	579	145	300	77	7
143	13890	147	580	146	300	77	7
144	13890	148	581	147	300	77	7
145	13890	149	582	148	300	77	7
146	13890	150	583	149	300	77	7
147	13890	151	584	150	300	77	7
148	13890	152	585	151	300	77	7
149	13890	153	586	152	300	77	7
150	13890	154	587	153	300	77	7
151	13890	155	588	154	300	77	7
152	13890	156	589	155	300	77	7
153	13890	157	590	156	300	77	7
154	13890	158	591	157	300	77	7
155	13890	159	592	158	300	77	7
156	13890	160	593	159	300	77	7
157	13890	161	594	160	300	77	7
158	13890	162	595	161	300	77	7
159	13890	163	596	162	300	77	7
160	13890	164	597	163	300	77	7
161	13890	165	598	164	300	77	7
162	13890	166	599	165	300	77	7
163	13890	167	600	166	300	77	7
164	13890	168	601	167	300	77	7
165	13890	169	602	168	300	77	7
166	13890	170	603	169	300	77	7
167	13890	171	604	170	300	77	7
168	13890	172	605	171	300	77	7
169	13890	173	606	172	300	77	7
170	13890	174	607	173	300	77	7
171	13890	175	608	174	300	77	7
172	13890	176	609	175	300	77	7
173	13890	177	610	176	300	77	7
174	13890	178	611	177	300	77	7
175	13890	179	612	178	300	77	7
176	13890	180	613	179	300	77	7
177	13890	181	614	180	300	77	7
178	13890	182	615	181	300	77	7
179	13890	183	616	182	300	77	7
180	13890	184	617	183	300	77	7
181	13890	185	618	184	300	77	7
182	13890	186	619	185	300	77	7
183	13890	187	620	186	300	77	7
184	13890	188	621	187	300	77	7
185	13890	189	622	188	300	77	7
186	13890	190	623	189	300	77	7
187	13890	191	624	190	300	77	7
188	13890	192	625	191	300	77	7
189	13890	193	626	192	300	77	7
190	13890	194	627	193	300	77	7
191	13890	195	628	194	300	77	7
192	13890	196	629	195	300	77	7
193	13890	197	630	196	300	77	7
194	13890	198	631	197	300	77	7
195	13890	199	632	198	300	77	7
196	13890	200	633	199	300	77	7
197	13890	201	634	200	300	77	7
198	13890	202	635	201	300	77	7
199	13890	203	636	202	300	77	7
200	13890	204	637	203	300	77	7
201	13890	205	638	204	300	77	7
202	13890	206	639	205	300	77	7
203	13890	207	640	206	300	77	7
204	13890	208	641	207	300	77	7
205	13890	209	642	208	300	77	7
206	13890	210	643	209	300	77	7
207	13890	211	644	210	300	77	7
208	13890	212	645	211	300	77	7
209	13890	213	646	212	300	77	7
210	13890	214	647	213	300	77	7
211	13890	215	648	214	300	77	7
212	13890	216	649	215	300	77	7
213	13890	217	650	216	300	77	7
214	13890	218	651	217	300	77	7
215	13890	219	652	218	300	77	7
216	13890	220	653	219	300	77	7
217	13890	221	654	220	300	77	7
218	13890	222	655	221	300	77	7
219	13890	223	656	222	300	77	7
220	13890	224	657	223	300	77	7
221	13890	225	658	224	300	77	7
222	13890	226	659	225	300	77	7
223	13890	227	660	226	300	77	7
224	13890	228	661	227	300	77	7
225	13890	229	662	228	300	77	7
226	13890	230	663	229	300	77	7
227	13890	231	664	230	300	77	7
228	13890	232	665	231	300	77	7
229	13890	233	666	232	300	77	7
230	13890	234	667	233	300	77	7
231	13890	235	668	234	300	77	7
232	13890	236	669	235	300	77	7
233	13890	237	670	236	300	77	7
234	13890	238	671	237	300	77	7
235	13890	239	672	238	300	77	7
236	13890	240	673	239	300	77	7
237	13890	241	674	240	300	77	7
238	13890	242	675	241	300	77	7
239	13890	243	676	242	300	77	7
240	13890	244	677	243	300	77	7
241	13890	245	678	244	300	77	7
242	13890	246	679	245	300	77	7
243	13890	247	680	246	300	77	7
244	13890	248	681	247	300	77	7
245	13890	249	682	248	300	77	7
246	13890	250	683	249	300	77	7
247	13890	251	684	250	300	77	7
248	13890	252	685	251	300	77	7
249	13890	253	686	252	300	77	7
250	13890	254	687	253	300	77	7
251	13890	255	688	254	300	77	7
252	13890	256	689	255	300	77	7
253	13890	257	690	256	300	77	7
254	13890	258	691	257	300	77	7
255	13890	259	692	258	300	77	7
256	13890	260	693	259	300	77	7
257	13890	261	694	260	300	77	7
258	13890	262	695	261	300	77	7
259	13890	263	696	262	300	77	7
260	13890	264	697	263	300	77	7
261	13890	265	698	264	300	77	7
262	13890	266	699	265	300	77	7
263	13890	267	700	266	300	77	7
264	13890	268	701	267	300	77	7
265	13890	269	702	268	300	77	7
266	13890	270	703	269	300	77	7
267	13890	271	704	270	300	77	7
268	13890	272	705	271	300	77	7
269	13890	273	706	272	300	77	7
270	13890	274	707	273	300	77	7
271	13890	275	708	274	300	77	7
272	13890	276	709	275	300	77	7
273	13890	277	710	276	300	77	7
274	13890	278	711	277	300	77	7
275	13890	279	712	278	300	77	7
276	13890	280	713	279	300	77	7
277	13890	281	714	280	300	77	7
278	13890	282	715	281	300	77	7
279	13890	283	716	282	300	77	7
280	13890	284	717	283	300	77	7
281	13890	285	718	284	300	77	7
282	13890	286	719	285	300	77	7
283	13890	287	720	286	30		

THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	PIPE NO.	MODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS
66	65	66	570.0	140.0	140.0
77	76	77	200.0	140.0	140.0
78	77	78	150.0	140.0	140.0
81	82	84	150.0	140.0	140.0
87	87	87	160.0	140.0	140.0
89	87	90	75.0	140.0	140.0
108	108	109	980.0	140.0	140.0
116	115	116	140.0	140.0	140.0
17	15	17	100.0	140.0	140.0
20	19	20	140.0	140.0	140.0
41	49	41	220.0	140.0	140.0
51	51	51	680.0	140.0	140.0
127	134	134	150.0	140.0	140.0
101	103	102	150.0	140.0	140.0
165	88	88	520.0	140.0	140.0

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	MODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (CLIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1	100.0	25.0	130.0	61.912	0.843	1.261	6.432	114.00
2	2	250.0	25.0	130.0	26.526	1.004	0.540	8.478	114.00
3	3	150.0	25.0	130.0	16.531	0.123	0.800	1.533	114.00
4	4	100.0	25.0	130.0	0.486	2.123	0.004	26.770	114.00
5	5	100.0	25.0	130.0	15.102	2.305	0.004	1.450	114.00
6	6	100.0	25.0	130.0	15.160	1.042	0.490	1.408	114.00
7	7	100.0	25.0	130.0	0.375	0.000	0.000	1.608	114.00
8	8	100.0	25.0	130.0	14.972	0.391	0.459	1.592	114.00
9	9	100.0	25.0	130.0	12.868	0.403	0.410	1.482	114.00
10	10	100.0	25.0	130.0	12.821	0.767	0.420	1.036	114.00
11	11	100.0	25.0	130.0	1.038	1.401	0.484	1.439	114.00
12	12	100.0	25.0	130.0	1.523	1.158	0.721	1.167	114.00
13	13	100.0	25.0	130.0	0.542	1.596	0.554	1.138	114.00
14	14	100.0	25.0	130.0	0.723	1.028	0.494	1.477	114.00
15	15	100.0	25.0	130.0	0.243	1.288	0.602	1.513	114.00
16	16	100.0	25.0	130.0	0.638	1.391	0.302	1.813	114.00
17	17	100.0	25.0	130.0	0.734	1.324	0.432	1.550	114.00
18	18	100.0	25.0	130.0	0.233	1.024	0.302	1.401	114.00
19	19	100.0	25.0	130.0	0.443	1.237	0.333	1.188	114.00
20	20	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
21	21	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
22	22	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
23	23	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
24	24	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
25	25	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
26	26	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
27	27	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
28	28	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
29	29	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
30	30	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
31	31	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
32	32	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
33	33	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
34	34	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
35	35	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
36	36	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
37	37	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
38	38	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
39	39	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
40	40	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
41	41	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
42	42	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
43	43	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
44	44	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
45	45	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
46	46	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
47	47	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
48	48	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
49	49	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
50	50	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
51	51	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
52	52	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
53	53	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
54	54	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
55	55	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
56	56	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
57	57	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
58	58	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
59	59	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
60	60	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
61	61	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
62	62	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
63	63	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
64	64	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
65	65	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
66	66	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
67	67	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
68	68	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
69	69	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
70	70	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
71	71	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
72	72	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
73	73	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
74	74	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
75	75	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
76	76	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
77	77	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
78	78	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
79	79	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
80	80	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
81	81	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
82	82	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
83	83	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
84	84	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
85	85	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
86	86	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
87	87	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
88	88	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
89	89	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
90	90	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
91	91	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
92	92	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
93	93	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
94	94	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
95	95	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
96	96	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
97	97	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
98	98	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
99	99	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00
100	100	100.0	25.0	130.0	0.233	1.324	0.333	1.468	114.00

[illegible]

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION NODES

INJECTION NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.0000	113.357	100.200	13.16
2	0.6250	110.474	99.870	10.60

23	1.28720	104.804	97.760	7.4222
99	1.00000	108.723	99.000	0.7775
95	0.50000	105.521	98.800	7.5513
96	0.00000	108.521	99.000	1.3282
7	0.75000	107.834	97.830	3.8272
99	1.25000	106.551	98.730	2.9272
99	1.00440	107.890	98.610	5.2529
100	1.77780	106.070	98.550	7.5759
101	0.25000	107.540	98.570	2.5799
102	0.00000	105.417	98.750	2.2529
103	2.13890	106.211	97.600	7.8782
104	1.16070	105.211	98.550	7.7255
105	1.75000	104.175	97.550	5.5707
106	0.48610	104.202	97.500	6.6757
107	0.96250	103.570	97.430	6.6057
108	0.19440	104.570	97.300	6.6083
109	0.98000	103.374	97.300	0.4031
110	0.00000	103.747	97.300	0.4835
111	0.00000	109.738	97.300	7.3529
112	0.00000	108.534	97.300	7.3027
113	0.00000	106.224	97.300	7.0274
114	0.00000	107.143	97.300	6.8705
115	0.00000	106.611	97.300	7.7160
116	0.00000	106.611	97.300	7.1099
117	0.00000	106.611	97.300	6.8334
118	0.00000	106.611	97.300	6.8334
119	0.00000	106.611	97.300	6.8334
120	0.00000	106.611	97.300	6.8334
121	0.00000	106.611	97.300	6.8334
122	0.00000	106.611	97.300	6.8334
123	0.00000	106.611	97.300	6.8334
124	0.00000	106.611	97.300	6.8334
125	0.00000	106.611	97.300	6.8334
126	0.00000	106.611	97.300	6.8334
127	0.00000	106.611	97.300	6.8334
128	0.00000	106.611	97.300	6.8334
129	0.00000	106.611	97.300	6.8334
130	0.00000	106.611	97.300	6.8334
131	0.00000	106.611	97.300	6.8334
132	0.00000	106.611	97.300	6.8334
133	0.00000	106.611	97.300	6.8334
134	0.00000	106.611	97.300	6.8334
135	0.00000	106.611	97.300	6.8334
136	0.00000	106.611	97.300	6.8334
137	0.00000	106.611	97.300	6.8334
138	0.00000	106.611	97.300	6.8334
139	0.00000	106.611	97.300	6.8334
140	0.00000	106.611	97.300	6.8334
141	0.00000	106.611	97.300	6.8334
142	0.00000	106.611	97.300	6.8334
143	0.00000	106.611	97.300	6.8334
144	0.00000	106.611	97.300	6.8334
145	0.00000	106.611	97.300	6.8334
146	0.00000	106.611	97.300	6.8334
147	0.00000	106.611	97.300	6.8334
148	0.00000	106.611	97.300	6.8334
149	0.00000	106.611	97.300	6.8334
150	0.00000	106.611	97.300	6.8334
151	0.00000	106.611	97.300	6.8334
152	0.00000	106.611	97.300	6.8334
153	0.00000	106.611	97.300	6.8334
154	0.00000	106.611	97.300	6.8334
155	0.00000	106.611	97.300	6.8334
156	0.00000	106.611	97.300	6.8334
157	0.00000	106.611	97.300	6.8334
158	0.00000	106.611	97.300	6.8334
159	0.00000	106.611	97.300	6.8334
160	0.00000	106.611	97.300	6.8334
161	0.00000	106.611	97.300	6.8334
162	0.00000	106.611	97.300	6.8334
163	0.00000	106.611	97.300	6.8334
164	0.00000	106.611	97.300	6.8334
165	0.00000	106.611	97.300	6.8334
166	0.00000	106.611	97.300	6.8334
167	0.00000	106.611	97.300	6.8334
168	0.00000	106.611	97.300	6.8334
169	0.00000	106.611	97.300	6.8334
170	0.00000	106.611	97.300	6.8334
171	0.00000	106.611	97.300	6.8334
172	0.00000	106.611	97.300	6.8334
173	0.00000	106.611	97.300	6.8334
174	0.00000	106.611	97.300	6.8334
175	0.00000	106.611	97.300	6.8334
176	0.00000	106.611	97.300	6.8334
177	0.00000	106.611	97.300	6.8334
178	0.00000	106.611	97.300	6.8334
179	0.00000	106.611	97.300	6.8334
180	0.00000	106.611	97.300	6.8334
181	0.00000	106.611	97.300	6.8334
182	0.00000	106.611	97.300	6.8334
183	0.00000	106.611	97.300	6.8334
184	0.00000	106.611	97.300	6.8334
185	0.00000	106.611	97.300	6.8334
186	0.00000	106.611	97.300	6.8334
187	0.00000	106.611	97.300	6.8334
188	0.00000	106.611	97.300	6.8334
189	0.00000	106.611	97.300	6.8334
190	0.00000	106.611	97.300	6.8334
191	0.00000	106.611	97.300	6.8334
192	0.00000	106.611	97.300	6.8334
193	0.00000	106.611	97.300	6.8334
194	0.00000	106.611	97.300	6.8334
195	0.00000	106.611	97.300	6.8334
196	0.00000	106.611	97.300	6.8334
197	0.00000	106.611	97.300	6.8334
198	0.00000	106.611	97.300	6.8334
199	0.00000	106.611	97.300	6.8334
200	0.00000	106.611	97.300	6.8334

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEME

QUANTITY (RS)	ROUGHNESS	RATE (/RS)	TOTAL LENGTH (METERS)	COST (RS)
225.00	130.00	112.00	850.00	95200.00
15.00	130.00	41.80	2875.00	711725.00
10.00	140.00	31.50	7005.00	220657.50
0.00	140.00	13.20	3475.00	458720.00
0.00	140.00	1	112550.00	125459.00
0.00	140.00	4.10	41280.00	225978.00
0.00	140.00	4.10	3775.00	15477.50
0.00	140.00	1.00	780.00	3120.00
TOTAL COST=				1525540.40

SCHEME 4

NALASA GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, KANPUR

THIS SYSTEM HAS ONE OVER HEAD TANK

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED AFTER 5 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CM)	ROUGHNESS	FLOW RATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADES (METERS)
1	1-2	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
2	1-3	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
3	1-4	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
4	1-5	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
5	1-6	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
6	1-7	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
7	1-8	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
8	1-9	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
9	1-10	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
10	1-11	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
11	1-12	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
12	1-13	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
13	1-14	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
14	1-15	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
15	1-16	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
16	1-17	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
17	1-18	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
18	1-19	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
19	1-20	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
20	1-21	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
21	1-22	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
22	1-23	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
23	1-24	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
24	1-25	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
25	1-26	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
26	1-27	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
27	1-28	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
28	1-29	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
29	1-30	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
30	1-31	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
31	1-32	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
32	1-33	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
33	1-34	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
34	1-35	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
35	1-36	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
36	1-37	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
37	1-38	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
38	1-39	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
39	1-40	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
40	1-41	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
41	1-42	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
42	1-43	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
43	1-44	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
44	1-45	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
45	1-46	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
46	1-47	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
47	1-48	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
48	1-49	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
49	1-50	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
50	1-51	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
51	1-52	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
52	1-53	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
53	1-54	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
54	1-55	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
55	1-56	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
56	1-57	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
57	1-58	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
58	1-59	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
59	1-60	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
60	1-61	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
61	1-62	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
62	1-63	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
63	1-64	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
64	1-65	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
65	1-66	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
66	1-67	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
67	1-68	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
68	1-69	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
69	1-70	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
70	1-71	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
71	1-72	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
72	1-73	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
73	1-74	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
74	1-75	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
75	1-76	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
76	1-77	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
77	1-78	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
78	1-79	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
79	1-80	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
80	1-81	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
81	1-82	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
82	1-83	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
83	1-84	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
84	1-85	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
85	1-86	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
86	1-87	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
87	1-88	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
88	1-89	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
89	1-90	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
90	1-91	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
91	1-92	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
92	1-93	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
93	1-94	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
94	1-95	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
95	1-96	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
96	1-97	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
97	1-98	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
98	1-99	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
99	1-100	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81
100	1-101	10.00	25.00	0.00	35.00	0.00	0.00	3.75	143.81

[illegible]

JUNCTION NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.00000	143.774	123.770	20.000
2	0.00000	143.516	122.840	20.733
3	0.00000	143.333	122.710	21.029
4	0.00000	143.118	122.510	20.633
5	0.00000	136.482	125.150	14.157
6	0.00000	136.935	125.020	9.037
7	0.19440	134.401	125.860	9.514
8	0.33060	134.319	124.310	9.345
9	0.12610	134.180	124.410	9.158
10	0.28330	134.038	123.530	8.355
11	0.17500	133.879	125.500	8.153
12	0.19440	133.751	124.420	8.286
13	0.20170	133.225	127.570	5.764
14	0.38690	143.715	125.810	5.444
15	0.35000	141.502	125.130	5.260
16	0.09720	140.559	125.950	5.469
17	0.00000	135.310	126.000	4.659
18	0.35000	138.505	127.870	11.04
19	0.19440	138.598	128.650	10.01
20	0.19440	137.921	129.210	7.61
21	0.19440	135.783	128.310	7.47

[illegible]

125	950
125	790
125	610
126	260
126	390
126	150
125	700
125	820
125	900
126	160
126	200
126	300
126	330
131	150
129	790
129	010
128	720
130	200
128	800
128	700
128	790
129	410
130	420
129	050
129	640
129	450
129	980
127	990
128	160
128	750
128	870
128	440
129	060
128	600
126	990
126	000
126	470
126	820
126	780
126	780
126	530
127	880
127	280
127	060
127	800
128	510
127	420
126	630
127	210
126	840
126	780
126	650
126	850
127	210
125	350
128	130
127	780
127	410
127	700
127	800
128	140
127	910
127	970
127	130
127	310
127	360
126	250
128	340
128	400

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7	00000000	126.210	7.56
8	00000000	126.340	8.79
9	00000000		
10	00000000		
11	00000000		
12	00000000		
13	00000000		
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33	00000000		
34	00000000		
35	00000000		
36	00000000		
37	00000000		
38	00000000		
39	00000000		
40	00000000		
41	00000000		
42	00000000		

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEME

DIAMETER (CMRS)	ROUGHNESS	RATE (/M(CRS)	TOTAL LENGTH (METERS)	COST (CMRS)
25.00	130.00	55.00	100.00	5500.00
20.00	130.00	40.00	125.00	5000.00
15.00	130.00	30.00	580.00	17400.00
10.00	130.00	25.00	6022.00	150550.00
5.00	130.00	17.00	2528.00	42984.00
2.5	130.00	12.70	3466.00	44152.00
2.5	130.00	7.70	3259.00	25113.00
2.5	130.00	4.70	3170.00	14869.00
2.5	130.00	4.15	2860.00	11869.00
2.5	130.00	4.10	8255.00	33845.50
TOTAL COST=				913214.70

THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS
87	72	80.0	3.0	140.0
77	75	275.0	3.2	140.0
102	78	100.0	3.5	140.0
97	84	120.0	3.2	140.0
103	86	115.0	3.2	140.0
112	105	170.0	3.2	140.0
123	108	15.0	3.0	140.0
135	113	65.0	3.5	140.0
152	113	10.0	3.5	140.0
167	114	60.0	3.2	140.0
178	115	60.0	3.2	140.0
189	117	152.0	3.5	140.0
197	118	160.0	3.5	140.0
198	119	60.0	3.5	140.0
199	120	90.0	3.5	140.0
200	121	125.0	3.5	140.0
201	122	150.0	3.5	140.0
202	123	150.0	3.5	140.0
203	124	150.0	3.5	140.0
204	125	150.0	3.5	140.0
205	126	150.0	3.5	140.0
206	127	150.0	3.5	140.0
207	128	150.0	3.5	140.0
208	129	150.0	3.5	140.0
209	130	150.0	3.5	140.0
210	131	150.0	3.5	140.0
211	132	150.0	3.5	140.0
212	133	150.0	3.5	140.0
213	134	150.0	3.5	140.0
214	135	150.0	3.5	140.0
215	136	150.0	3.5	140.0
216	137	150.0	3.5	140.0
217	138	150.0	3.5	140.0
218	139	150.0	3.5	140.0
219	140	150.0	3.5	140.0
220	141	150.0	3.5	140.0
221	142	150.0	3.5	140.0
222	143	150.0	3.5	140.0
223	144	150.0	3.5	140.0
224	145	150.0	3.5	140.0
225	146	150.0	3.5	140.0
226	147	150.0	3.5	140.0
227	148	150.0	3.5	140.0
228	149	150.0	3.5	140.0
229	150	150.0	3.5	140.0
230	151	150.0	3.5	140.0
231	152	150.0	3.5	140.0
232	153	150.0	3.5	140.0
233	154	150.0	3.5	140.0
234	155	150.0	3.5	140.0
235	156	150.0	3.5	140.0
236	157	150.0	3.5	140.0
237	158	150.0	3.5	140.0
238	159	150.0	3.5	140.0
239	160	150.0	3.5	140.0
240	161	150.0	3.5	140.0
241	162	150.0	3.5	140.0
242	163	150.0	3.5	140.0
243	164	150.0	3.5	140.0
244	165	150.0	3.5	140.0
245	166	150.0	3.5	140.0
246	167	150.0	3.5	140.0
247	168	150.0	3.5	140.0
248	169	150.0	3.5	140.0
249	170	150.0	3.5	140.0
250	171	150.0	3.5	140.0
251	172	150.0	3.5	140.0
252	173	150.0	3.5	140.0
253	174	150.0	3.5	140.0
254	175	150.0	3.5	140.0
255	176	150.0	3.5	140.0
256	177	150.0	3.5	140.0
257	178	150.0	3.5	140.0
258	179	150.0	3.5	140.0
259	180	150.0	3.5	140.0
260	181	150.0	3.5	140.0
261	182	150.0	3.5	140.0
262	183	150.0	3.5	140.0
263	184	150.0	3.5	140.0
264	185	150.0	3.5	140.0
265	186	150.0	3.5	140.0
266	187	150.0	3.5	140.0
267	188	150.0	3.5	140.0
268	189	150.0	3.5	140.0
269	190	150.0	3.5	140.0
270	191	150.0	3.5	140.0
271	192	150.0	3.5	140.0
272	193	150.0	3.5	140.0
273	194	150.0	3.5	140.0
274	195	150.0	3.5	140.0
275	196	150.0	3.5	140.0
276	197	150.0	3.5	140.0
277	198	150.0	3.5	140.0
278	199	150.0	3.5	140.0
279	200	150.0	3.5	140.0
280	201	150.0	3.5	140.0
281	202	150.0	3.5	140.0
282	203	150.0	3.5	140.0
283	204	150.0	3.5	140.0
284	205	150.0	3.5	140.0
285	206	150.0	3.5	140.0
286	207	150.0	3.5	140.0
287	208	150.0	3.5	140.0
288	209	150.0	3.5	140.0
289	210	150.0	3.5	140.0
290	211	150.0	3.5	140.0
291	212	150.0	3.5	140.0
292	213	150.0	3.5	140.0
293	214	150.0	3.5	140.0
294	215	150.0	3.5	140.0
295	216	150.0	3.5	140.0
296	217	150.0	3.5	140.0
297	218	150.0	3.5	140.0
298	219	150.0	3.5	140.0
299	220	150.0	3.5	140.0
300	221	150.0	3.5	140.0
301	222	150.0	3.5	140.0
302	223	150.0	3.5	140.0
303	224	150.0	3.5	140.0
304	225	150.0	3.5	140.0
305	226	150.0	3.5	140.0
306	227	150.0	3.5	140.0
307	228	150.0	3.5	140.0
308	229	150.0	3.5	140.0
309	230	150.0	3.5	140.0
310	231	150.0	3.5	140.0
311	232	150.0	3.5	140.0
312	233	150.0	3.5	140.0
313	234	150.0	3.5	140.0
314	235	150.0	3.5	140.0
315	236	150.0	3.5	140.0
316	237	150.0	3.5	140.0
317	238	150.0	3.5	140.0
318	239	150.0	3.5	140.0
319	240	150.0	3.5	140.0
320	241	150.0	3.5	140.0
321	242	150.0	3.5	140.0
322	243	150.0	3.5	140.0
323	244	150.0	3.5	140.0
324	245	150.0	3.5	140.0
325	246	150.0	3.5	140.0
326	247	150.0	3.5	140.0
327	248	150.0	3.5	140.0
328	249	150.0	3.5	140.0
329	250	150.0	3.5	140.0
330	251	150.0	3.5	140.0
331	252	150.0	3.5	140.0
332	253	150.0	3.5	140.0
333	254	150.0	3.5	140.0
334	255	150.0	3.5	140.0
335	256	150.0	3.5	140.0
336	257	150.0	3.5	140.0
337	258	150.0	3.5	140.0
338	259	150.0	3.5	140.0
339	260	150.0	3.5	140.0
340	261	150.0	3.5	140.0
341	262	150.0	3.5	140.0
342	263	150.0	3.5	140.0
343	264	150.0	3.5	140.0
344	265	150.0	3.5	140.0
345	266	150.0	3.5	140.0
346	267	150.0	3.5	140.0
347	268	150.0	3.5	140.0
348	269	150.0	3.5	140.0
349	270	150.0	3.5	140.0
350	271	150.0	3.5	140.0
351	272	150.0	3.5	140.0
352	273	150.0	3.5	140.0
353	274	150.0	3.5	140.0
354	275	150.0	3.5	140.0
355	276	150.0	3.5	140.0
356	277	150.0	3.5	140.0
357	278	150.0	3.5	140.0
358	279	150.0	3.5	140.0
359	280	150.0	3.5	140.0
360	281	150.0	3.5	140.0
361	282	150.0	3.5	140.0
362	283	150.0	3.5	140.0
363	284	150.0	3.5	140.0
364	285	150.0	3.5	140.0
365	286	150.0	3.5	140.0
366	287	150.0	3.5	140.0
367	288	150.0	3.5	140.0
368	289	150.0	3.5	140.0
369	290	150.0	3.5	140.0
370	291	150.0	3.5	140.0
371	292	150.0	3.5	140.0
372	293	150.0	3.5	140.0
373	294	150.0	3.5	140.0
374	295	150.0	3.5	140.0
375	296	150.0	3.5	140.0
376	297	150.0	3.5	140.0
377	298	150.0	3.5	140.0
378	299	150.0	3.5	140.0
379	300	150.0	3.5	140.0
380	301	150.0	3.5	140.0
381	302	150.0	3.5	140.0
382	303	150.0	3.5	140.0
383	304	150.0	3.5	140.0
384	305	150.0	3.5	140.0
385	306	150.0	3.5	140.0
386	307	150.0	3.5	140.0
387	308	150.0	3.5	140.0
388	309	150.0	3.5	140.0
389	310	150.0	3.5	140.0
390	311	150.0	3.5	140.0
391	312	150.0	3.5	140.0
392	313	150.0	3.5	140.0
393	314	150.0	3.5	140.0
394	315	150.0	3.5	140.0
395	316	150.0	3.5	140.0
396	317	150.0	3.5	140.0
397	318	150.0	3.5	140.0
398	319	150.0	3.5	140.0
399	320	150.0	3.5	140.0
400	321	150.0	3.5	140.0
401	322	150.0	3.5	140.0
402	323	150.0	3.5	140.0
403	324	150.0	3.5	140.0
404	325	150.0	3.5	140.0
405	326	150.0	3.5	140.0
406	327	150.0	3.5	140.0
407	328	150.0	3.5	140.0
408	329	150.0	3.5	140.0
409	330	150.0	3.5	140.0
410	331	150.0	3.5	140.0
411	332	150.0	3.5	140.0
412	333	150.0	3.5	140.0
413	334	150.0	3.5	140.0
414	335	150.0	3.5	140.0
415	336	150.0	3.5	140.0
416	337	150.0	3.5	140.0
417	338	150.0	3.5	140.0
418	339	150.0	3.5	140.0
419	340	150.0	3.5	140.0
420	341	150.0	3.5	140.0
421	342	150.0	3.5	140.0
422	343	150.0	3.5	140.0
423	344	150.0	3.5	140.0
424	345	150.0	3.5	140.0
425	346	150.0	3.5	140.0
426	347	150.0	3.5	140.0
427	348	150.0	3.5	140.0
428	349	150.0	3.5	140.0
429	350	150.0	3.5	140.0
430	351	150.0</		

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0.097	0.194	0.150	0.194	0.138	0.008	0.291	0.049	0.671	0.418	0.194	0.194	0.418	0.253	0.127	0.194	0.175	0.722	0.292	0.319	0.389	0.520	0.161	1.695	1.699	2.456	2.716	0.716	0.113	0.196	0.175	0.058	0.117	0.292	1.119	2.797	1.114	0.684	0.491	0.373	0.208	0.567	0.567	1.414	2.244	1.236	1.267	0.057	2.364	3.857	1.834	1.681	0.698	1.566	1.566	1.390	1.391	1.145	1.566	0.812	1.597	3.455	3.459	0.658	0.783	0.523	1.265	1.655	1.111	1.06
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JUNCTION NO.	DEMAND (LIT/SEC)	GRADE LINE (FEET)	ELEVATION (FEET)	PRESSURE (FEET)
1	0.00000	143.616	123.770	20.000
2	0.25300	143.330	122.890	20.733
3	0.33060	143.195	122.240	21.099
4	0.00000	142.117	122.710	20.149
5	0.00000	139.481	125.510	16.333
6	0.00000	136.401	125.790	11.147
7	0.00000	134.318	125.030	9.277
8	0.00000	134.227	125.290	9.277
9	0.00000	134.179	124.860	9.277
10	0.00000	134.057	124.310	9.277
11	0.00000	133.878	122.410	11.645
12	0.00000	133.737	123.500	11.645
13	0.00000	133.525	123.420	10.232
14	0.00000	133.255	123.570	9.459
15	0.00000	133.157	123.130	9.500
16	0.00000	131.507	125.340	21.541
17	0.00000	141.593	126.000	21.541
18	0.00000	135.314	126.000	10.645
19	0.00000	138.502	126.000	10.645
20	0.00000	137.502	128.000	10.645
21	0.00000	135.225	128.000	10.645
22	0.00000	135.787	128.000	10.645
23	0.00000	135.713	128.000	10.645
24	0.00000	135.033	128.000	10.645
25	0.00000	134.258	128.000	10.645
26	0.00000	134.217	128.000	10.645
27	0.00000	134.103	128.000	10.645
28	0.00000	134.120	128.000	10.645
29	0.00000	133.642	128.000	10.645
30	0.00000	133.883	128.000	10.645
31	0.00000	133.883	128.000	10.645
32	0.00000	133.883	128.000	10.645
33	0.00000	133.883	128.000	10.645
34	0.00000	133.883	128.000	10.645
35	0.00000	133.883	128.000	10.645
36	0.00000	133.883	128.000	10.645
37	0.00000	133.883	128.000	10.645
38	0.00000	133.883	128.000	10.645
39	0.00000	133.883	128.000	10.645
40	0.00000	133.883	128.000	10.645
41	0.00000	133.883	128.000	10.645
42	0.00000	133.883	128.000	10.645
43	0.00000	133.883	128.000	10.645
44	0.00000	133.883	128.000	10.645
45	0.00000	133.883	128.000	10.645
46	0.00000	133.883	128.000	10.645
47	0.00000	133.883	128.000	10.645
48	0.00000	133.883	128.000	10.645
49	0.00000	133.883	128.000	10.645
50	0.00000	133.883	128.000	10.645
51	0.00000	133.883	128.000	10.645
52	0.00000	133.883	128.000	10.645
53	0.00000	133.883	128.000	10.645
54	0.00000	133.883	128.000	10.645
55	0.00000	133.883	128.000	10.645
56	0.00000	133.883	128.000	10.645
57	0.00000	133.883	128.000	10.645
58	0.00000	133.883	128.000	10.645
59	0.00000	133.883	128.000	10.645
60	0.00000	133.883	128.000	10.645
61	0.00000	133.883	128.000	10.645
62	0.00000	133.883	128.000	10.645
63	0.00000	133.883	128.000	10.645
64	0.00000	133.883	128.000	10.645
65	0.00000	133.883	128.000	10.645
66	0.00000	133.883	128.000	10.645
67	0.00000	133.883	128.000	10.645
68	0.00000	133.883	128.000	10.645
69	0.00000	133.883	128.000	10.645
70	0.00000	133.883	128.000	10.645
71	0.00000	133.883	128.000	10.645
72	0.00000	133.883	128.000	10.645
73	0.00000	133.883	128.000	10.645
74	0.00000	133.883	128.000	10.645
75	0.00000	133.883	128.000	10.645
76	0.00000	133.883	128.000	10.645
77	0.00000	133.883	128.000	10.645
78	0.00000	133.883	128.000	10.645
79	0.00000	133.883	128.000	10.645
80	0.00000	133.883	128.000	10.645
81	0.00000	133.883	128.000	10.645
82	0.00000	133.883	128.000	10.645
83	0.00000	133.883	128.000	10.645
84	0.00000	133.883	128.000	10.645
85	0.00000	133.883	128.000	10.645
86	0.00000	133.883	128.000	10.645
87	0.00000	133.883	128.000	10.645
88	0.00000	133.883	128.000	10.645
89	0.00000	133.883	128.000	10.645
90	0.00000	133.883	128.000	10.645
91	0.00000	133.883	128.000	10.645
92	0.00000	133.883	128.000	10.645
93	0.00000	133.883	128.000	10.645
94	0.00000	133.883	128.000	10.645
95	0.00000	133.883	128.000	10.645
96	0.00000	133.883	128.000	10.645
97	0.00000	133.883	128.000	10.645
98	0.00000	133.883	128.000	10.645
99	0.00000	133.883	128.000	10.645
100	0.00000	133.883	128.000	10.645

US\$		(/M(RS))	(METERS)	(RS)
22500000	00000000	550000	10000	500000
22500000	30000000	400000	82500	3700000
22500000	30000000	300000	83000	2400000
22500000	30000000	250000	85220	1130500
22500000	30000000	170000	30830	328040
22500000	30000000	120000	30830	287490
22500000	30000000	70000	30670	226660
22500000	30000000	70000	32380	139160
22500000	30000000	40000	30610	160140
22500000	30000000	40000	30550	251740
22500000	30000000	40000	61400	251740
TOTAL COST#				93862.95

SCHEME 5

PAJAPUR GROUP OF VILLAGES RURAL WATER SUPPLY SCHEME, KANPUR

THIS SYSTEM HAS ONE OVER HEAD TANK

THIS SYSTEM HAS 86 PIPES, 60 JUNCTIONS, 26 LOOPS AND 1 FIXED GRADE NODE

THESE ARE THE RESULTS FOR THE FLOWS IN PIPES

THE FOLLOWING RESULTS ARE OBTAINED AFTER 6 TRIALS

PIPE NO.	NODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
1	1 2	100.00	20.00	130.00	22.389	0.261	0.713	22.2	899
2	1 3	150.00	10.00	130.00	23.444	0.367	0.807	22.2	899
3	1 4	35.00	10.00	130.00	23.198	0.231	0.407	1.3	307
4	1 5	30.00	10.00	130.00	23.323	0.101	0.359	1.1	783
5	1 6	30.00	10.00	130.00	15.331	0.152	0.488	1.1	437
6	1 7	30.00	10.00	130.00	14.538	0.040	0.466	1.1	327
7	1 8	30.00	10.00	130.00	13.090	0.232	0.741	1.1	635
8	1 9	30.00	10.00	130.00	12.544	0.151	0.710	1.1	325
9	1 10	30.00	10.00	130.00	12.136	0.169	0.688	1.1	554
10	1 11	30.00	10.00	130.00	11.654	0.270	0.663	1.1	785
11	1 12	30.00	10.00	130.00	9.853	0.170	0.535	1.1	514
12	1 13	77.00	10.00	130.00	8.274	0.170	0.468	1.1	384
13	1 14	107.50	10.00	130.00	7.194	0.218	0.407	1.1	592
14	1 15	175.00	10.00	130.00	7.370	0.437	0.301	1.1	327
15	1 16	155.00	10.00	130.00	7.926	0.303	0.273	1.1	235
16	1 17	130.00	10.00	130.00	6.690	0.223	0.221	1.1	235
17	1 18	130.00	10.00	130.00	6.334	0.034	0.211	1.1	257
18	1 19	130.00	10.00	130.00	2.679	0.034	0.233	1.1	257
19	1 20	130.00	10.00	130.00	1.545	0.098	0.164	1.1	257
20	1 21	130.00	10.00	130.00	0.900	0.121	0.164	1.1	257
21	1 22	130.00	10.00	130.00	2.215	0.252	0.164	1.1	257
22	1 23	130.00	10.00	130.00	2.778	0.189	0.164	1.1	257
23	1 24	130.00	10.00	130.00	2.421	0.189	0.164	1.1	257
24	1 25	130.00	10.00	130.00	2.160	0.368	0.164	1.1	257
25	1 26	130.00	10.00	130.00	1.802	0.331	0.164	1.1	257
26	1 27	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
27	1 28	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
28	1 29	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
29	1 30	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
30	1 31	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
31	1 32	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
32	1 33	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
33	1 34	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
34	1 35	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
35	1 36	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
36	1 37	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
37	1 38	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
38	1 39	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
39	1 40	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
40	1 41	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
41	1 42	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
42	1 43	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
43	1 44	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
44	1 45	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
45	1 46	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
46	1 47	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
47	1 48	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
48	1 49	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
49	1 50	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
50	1 51	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
51	1 52	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
52	1 53	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
53	1 54	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
54	1 55	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
55	1 56	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
56	1 57	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
57	1 58	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
58	1 59	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
59	1 60	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
60	1 61	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
61	1 62	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
62	1 63	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
63	1 64	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
64	1 65	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
65	1 66	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
66	1 67	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
67	1 68	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
68	1 69	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
69	1 70	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
70	1 71	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
71	1 72	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
72	1 73	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
73	1 74	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
74	1 75	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
75	1 76	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
76	1 77	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
77	1 78	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
78	1 79	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
79	1 80	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
80	1 81	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
81	1 82	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
82	1 83	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
83	1 84	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
84	1 85	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
85	1 86	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257
86	1 87	130.00	10.00	130.00	1.027	0.158	0.164	1.1	257

JUNCTION NO.	DEMAND (LIT/SEC)	GRADE LINE (METERS)	ELEVATION (METERS)	PRESSURE (METERS)
1	0.000000	110.730	99.000	11.74
2	0.000000	110.372	98.525	11.79
3	0.000000	110.314	98.335	11.74
4	0.000000	110.045	99.725	10.47
5	0.000000	109.593	99.735	10.79
6	0.000000	109.296	99.855	9.78
7	0.000000	109.237	109.000	9.44
8	0.000000	110.191	109.040	9.29
9	0.000000	110.000	100.120	10.00
10	0.000000	109.908	100.115	9.79
11	0.000000	109.852	100.655	8.51
12	0.000000	109.817	100.935	8.41
13	0.000000	110.041	99.325	11.64
14	0.000000	109.970	99.920	11.50
15	0.000000	109.817	99.475	12.40
16	0.000000	109.762	99.575	10.29
17	0.000000	109.833	99.935	11.26
18	0.000000	109.471	99.935	10.71
19	0.000000	109.680	100.840	9.54
20	0.000000	109.488	100.270	10.84
21	0.000000	109.002	100.090	9.29
22	0.000000	109.110	100.980	9.07
23	0.000000	109.092	99.915	9.13
24	0.000000	108.871	99.050	10.45
25	0.000000	108.725	97.885	10.82
26	0.000000	108.427	98.335	10.53
27	0.000000	108.483	98.335	10.53
28	0.000000	108.705	99.325	10.00
29	0.000000	109.066	100.325	9.50
30	0.000000	109.739	100.375	9.50
31	0.000000	107.331	99.700	10.30
32	0.000000	107.100	99.700	10.30
33	0.000000	106.844	99.650	10.35
34	0.000000	106.573	97.330	10.23
35	0.000000	106.073	97.335	10.53
36	0.000000	107.056	96.325	10.79
37	0.000000	107.110	96.325	10.79
38	0.000000	107.087	99.295	10.79
39	0.000000	109.705	99.575	10.24
40	0.000000	109.190	98.440	10.50
41	0.000000	109.030	97.060	10.85
42	0.000000	108.914	97.285	11.74
43	0.000000	109.026	97.285	11.74
44	0.000000	109.104	99.955	12.04
45	0.000000	109.281	97.805	10.53

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEME

DIAMETER (CMS)	ROUGHNESS	RATE (/M(RS))	TOTAL LENGTH (METERS)	COST (RS)
20.0	130.0	40.00	310.0	12400.00
15.5	130.0	28.00	1477.0	41474.16
12.0	130.0	24.00	235.0	5640.00
10.0	130.0	19.00	1457.0	27683.00
8.5	130.0	16.50	3465.0	57172.50
6.5	130.0	10.50	1265.0	13282.50
5.0	140.0	7.60	1315.0	9994.00
4.2	140.0	5.60	1495.0	8372.00
3.2	140.0	4.10	465.0	1906.50
TOTAL COST=				177924.66

THE FOLLOWING CHANGES IN SYSTEM DATA ARE MADE

PIPE NO.	MODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS
7	45	180.00	5.00	140.00
23	45	170.00	5.00	140.00
34	11	240.00	5.00	140.00
40	22	120.00	5.00	140.00
57	23	120.00	5.00	140.00
67	23	120.00	5.00	140.00
70	23	120.00	5.00	140.00
72	23	120.00	5.00	140.00
73	23	120.00	5.00	140.00
74	23	120.00	5.00	140.00
75	23	120.00	5.00	140.00
76	23	120.00	5.00	140.00
77	23	120.00	5.00	140.00
78	23	120.00	5.00	140.00
79	23	120.00	5.00	140.00
80	23	120.00	5.00	140.00
81	23	120.00	5.00	140.00
82	23	120.00	5.00	140.00
83	23	120.00	5.00	140.00
84	23	120.00	5.00	140.00
85	23	120.00	5.00	140.00
86	23	120.00	5.00	140.00
87	23	120.00	5.00	140.00
88	23	120.00	5.00	140.00
89	23	120.00	5.00	140.00
90	23	120.00	5.00	140.00
91	23	120.00	5.00	140.00
92	23	120.00	5.00	140.00
93	23	120.00	5.00	140.00
94	23	120.00	5.00	140.00
95	23	120.00	5.00	140.00
96	23	120.00	5.00	140.00
97	23	120.00	5.00	140.00
98	23	120.00	5.00	140.00
99	23	120.00	5.00	140.00
100	23	120.00	5.00	140.00

THE FOLLOWING RESULTS ARE OBTAINED AFTER 2 TRIALS

PIPE NO.	MODE NUMBERS	LENGTH (METERS)	DIAMETER (CMS)	ROUGHNESS	FLOWRATE (LIT/SEC)	HEAD LOSS (METERS)	VELOCITY (M/SEC)	HL/1000	FIXED GRADE (METERS)
7	45	180.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
23	45	170.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
34	11	240.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
40	22	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
57	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
67	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
70	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
72	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
73	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
74	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
75	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
76	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
77	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
78	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
79	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
80	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
81	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
82	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
83	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
84	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
85	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
86	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
87	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
88	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
89	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
90	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
91	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
92	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
93	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
94	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
95	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
96	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
97	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
98	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
99	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00
100	23	120.00	5.00	140.00	22.384	0.367	0.713	2.05	111.00

[illegible]

THESE ARE THE RESULTS FOR THE PRESSURES AT JUNCTION MODES

JUNCTION NO.	DEMAND	GRADE LINE	ELEVATION	PRESSURE
1	0.00	100.00	100.00	100.00
2	0.00	100.00	100.00	100.00
3	0.00	100.00	100.00	100.00
4	0.00	100.00	100.00	100.00
5	0.00	100.00	100.00	100.00
6	0.00	100.00	100.00	100.00
7	0.00	100.00	100.00	100.00
8	0.00	100.00	100.00	100.00
9	0.00	100.00	100.00	100.00
10	0.00	100.00	100.00	100.00
11	0.00	100.00	100.00	100.00
12	0.00	100.00	100.00	100.00
13	0.00	100.00	100.00	100.00
14	0.00	100.00	100.00	100.00
15	0.00	100.00	100.00	100.00
16	0.00	100.00	100.00	100.00
17	0.00	100.00	100.00	100.00
18	0.00	100.00	100.00	100.00
19	0.00	100.00	100.00	100.00
20	0.00	100.00	100.00	100.00
21	0.00	100.00	100.00	100.00
22	0.00	100.00	100.00	100.00
23	0.00	100.00	100.00	100.00
24	0.00	100.00	100.00	100.00
25	0.00	100.00	100.00	100.00
26	0.00	100.00	100.00	100.00
27	0.00	100.00	100.00	100.00
28	0.00	100.00	100.00	100.00
29	0.00	100.00	100.00	100.00
30	0.00	100.00	100.00	100.00
31	0.00	100.00	100.00	100.00
32	0.00	100.00	100.00	100.00
33	0.00	100.00	100.00	100.00
34	0.00	100.00	100.00	100.00
35	0.00	100.00	100.00	100.00
36	0.00	100.00	100.00	100.00
37	0.00	100.00	100.00	100.00
38	0.00	100.00	100.00	100.00
39	0.00	100.00	100.00	100.00
40	0.00	100.00	100.00	100.00
41	0.00	100.00	100.00	100.00
42	0.00	100.00	100.00	100.00
43	0.00	100.00	100.00	100.00
44	0.00	100.00	100.00	100.00
45	0.00	100.00	100.00	100.00
46	0.00	100.00	100.00	100.00
47	0.00	100.00	100.00	100.00
48	0.00	100.00	100.00	100.00
49	0.00	100.00	100.00	100.00
50	0.00	100.00	100.00	100.00
51	0.00	100.00	100.00	100.00
52	0.00	100.00	100.00	100.00
53	0.00	100.00	100.00	100.00
54	0.00	100.00	100.00	100.00
55	0.00	100.00	100.00	100.00
56	0.00	100.00	100.00	100.00
57	0.00	100.00	100.00	100.00
58	0.00	100.00	100.00	100.00
59	0.00	100.00	100.00	100.00
60	0.00	100.00	100.00	100.00
61	0.00	100.00	100.00	100.00
62	0.00	100.00	100.00	100.00
63	0.00	100.00	100.00	100.00
64	0.00	100.00	100.00	100.00
65	0.00	100.00	100.00	100.00
66	0.00	100.00	100.00	100.00
67	0.0			

JUNCTION NO.
 DEMAND
 (CU./SEC.)
 GRADE
 (FEET)
 GRADE
 (FEET)
 ELEVATION
 (FEET)
 PRESSURE
 (FEET)

43	0.77778	106.710	97.910	8.67
44	0.77778	106.665	98.000	8.62
45	1.74999	106.011	98.365	8.52
46	1.16669	106.374	97.850	8.44
47	1.38889	106.525	97.325	8.38
48	1.54449	106.646	97.325	8.33
49	1.77778	106.706	96.245	8.21
50	1.58333	106.672	96.575	8.14
51	1.50000	107.128	97.575	8.09
52	1.50000	107.033	98.140	8.05
53	1.58333	106.799	97.060	7.93
54	1.48611	106.879	97.245	7.86
55	1.48611	106.949	97.285	7.83
56	1.09722	107.071	96.965	7.73
57	1.09722	107.071	96.965	7.66
58	0.19444	107.540	97.803	7.50
59	0.25000			7.43

THE FOLLOWING IS THE COST STATEMENT OF THE SCHEM

DIAMETER (INCHES)	ROUGHNESS	RATE (/HRS)	TOTAL LENGTH (FEET)	COST (HRS)
20.00	130.00	40.00	240.00	9.60
18.00	130.00	28.00	154.00	4.31
16.00	130.00	28.00	135.00	3.79
14.00	130.00	16.50	175.00	2.88
12.00	130.00	17.60	262.00	4.65
10.00	140.00	7.50	153.00	1.15
8.00	140.00	4.10	106.00	0.43
TOTAL COST				156.58